

State of Hawaii, Department of Health, Clean Water Branch

NPDES Form C

Application for HAR, Chapter 11-55 - NPDES Individual Permit Authorizing Discharges of Storm Water Associated With Construction Activities (as defined in 40 CFR §§122.26(b)(14)(x) and 122.26(b)(15)(i))

All sections of this form MUST be completed for National Pollutant Discharge Elimination System (NPDES) Permit compliance.

C.1 – General Information

You are required to fulfill all requirements and <u>check the box</u> below. If you do not check the box, your application will be considered incomplete, and the CWB may deny your request for NPDES permit coverage with prejudice.

\square *I certify that:*

- My Storm Water Pollution Prevention Plan (SWPPP) was prepared in accordance with HAR, Chapter 11-55, Appendix C, Section 7.
- I will comply with all terms, conditions, and requirements in HAR Chapter 11-55, Appendix C.
- I will implement, operate, and maintain my SWPPP to ensure that storm water discharges associated with construction activities will not violate HAR, Chapter 11-54; HAR, Chapter 11-55; and HAR, Chapter 11-55, Appendix C.

C.2 - Existing Pollution Sources/ History of Land Use

Describe the history of land use at the existing Facility/Project site: The project is located along Kamehameha Highway (State Route 83) near Milepost 21 in the Hau'ula, Island of O'ahu, Hawai'i, and includes replacing the existing 1932 Kaipapa'u Stream Bridge with a new bridge and maintenance dredging and bank stabilization of the Kaipapa'u Stream. The bridge serves northbound traffic (toward Kahuku) and southbound traffic (toward Kane'ohe) on Kamehameha Highway. The bridge structure has two 40-foot spans and is constructed from reinforced concrete with a wooden pedestrian walkway attached to the mauka (west) side of the bridge. The bridge crosses Kaipapa'u Stream approximately 300 feet upstream from coastal marine waters. Beneath and makai of the Kaipapa'u Stream Bridge the stream is tidally influenced. Lands surrounding the bridge are single family residential and commercial in character and are privately owned. Parcels immediately surrounding Kaipapa'u Stream Bridge are single family residential.

Determine if the existing Facility/Project site may contain any existing pollution source(s) by using the following references. Place a check next to all references you utilized to determine existing pollution source(s). You are required to check at least one reference.

Ø a.	DOH, Solid and Hazardous Waste Branch-Hawaii Underground Storage Tank- Leaking
	Underground Storage Tank database
$\square b$.	DOH, Hazard Evaluation and Emergency Response Office records
\square c.	Phase I and/or Phase II Environmental Site Assessments, as applicable
$\Box d$.	Recent site inspections
\square e.	Past land use history
□ f.	Soil sampling data, if available
□ g.	Other (specify):

Describe any existing pollution source(s) identified in the references you checked above:

There are no pre-existing conditions other than soils that would result in potential for adverse impacts due to construction storm water runoff. The following practices will be employed to prevent discharges due to erosion: (1) adherence to the Hawaii Department of Transportation (HDOT) Construction Best Management Plan; and (2) structural measures including the use of temporary BMPs shall be placed to divert storm flows around materials storage locations. PVC sheet plastic or similar material shall also be placed to prevent inadvertent mixing of stored materials with storm water. Where mixing of storm water with soils cannot be avoided use of silt fencing and/or vegetative controls including grassing and hydromulching will be employed.

Describe any corrective measures that have been undertaken for any existing pollution source(s): N/A

Note: You are required to contact the Department of Health, Office of Hazard Evaluation and Emergency Response at (808) 586-4249 if contaminated soil or groundwater is known to be present at your project site.

C.3 - Construction Site Estimates Please provide the following estimates for the construction site. Total project area including areas to be left undisturbed: 1.6 acres Construction site area to be disturbed including storage and staging areas: 1.6 acres Impervious area before construction: 0.78 acres Impervious area after construction: 0.84 acres

C.4 - Quantity of Storm Water Runoff

Estimate the quantity of storm water runoff during construction when the greatest and/or maximum area of disturbance occurs. Provide the supporting calculations in an attachment or insert in this section. See Attachment A-3, Quantity of Storm Water Discharge Calculations

7.92 Cubic Feet per Second (CFS)

C.5 - Soil Characterization

Describe the nature of the soil on the project site (including the potential to encounter contaminated soil) and the nature of the fill material to be used:

The area surrounding Kaipapa'u Stream as it empties into the Pacific Ocean belongs to four soil series: Jaucas, Kawaihapai, Lolekaa, and Waikane. See Attachment A-1, Figure 3, Soil Classifications.

The Jaucas series consists of excessively drained, calcareous soils that occur as narrow strips on coastal plains, adjacent to the ocean.

JaC - Jaucas Sand, 0 to 15 percent slopes - Jaucas sand consists of excessively drained, calcareous soils. In most places the slope does not exceed 7%. Permeability is rapid. Runoff is slow to very slow. The hazard of water erosion is slight, however wind erosion is a severe hazard where vegetation has been removed. Jaucas sand deposits are associated with traditional Hawaiian burial practices and are commonly found to contain archaeological deposits.

The Kawaihapai series consists of well-drained soils in drainageways and on alluvial fans on the coastal plains of Oʻahu. These soils formed the alluvium derived from basic igneous rock in humid uplands.

KIA - Kawaihapai clay loam, 0 to 2 percent slopes - Kawaihapai soils consist of well drained soils in drainageways and on alluvial fans on the coastal plains. Permeability in this soil type is moderate, runoff is slow, and the erosion hazard is no more than slight.

<u>KiaB - Kawaihapai stony clay loam, 0 to 2 percent slopes - runoff is slow and erosion hazard is slight.</u> This soil type is prevalent on the banks of the Kaipapa'u Stream.

KIB - Kawaihapai clay loam, 2 to 6 percent slopes - Kawaihapai soils consists of soils where runoff is slow and the erosion hazard is slight. This soil type is found to the north of the Kaipapa'u Bridge, between Kamehameha Highway and the Pacific Ocean.

The Lolekaa series consists of well-drained soils on fans and terraces on the windward side of the island of O'ahu. These soils developed in old, gravelly colluvium and alluvium.

<u>LoB</u> - <u>Lolekaa silty clay, 3 to 8 percent slopes - This soil is found in terraces and fans.</u> <u>Runoff is slow, and erosion hazard is slight.</u>

<u>LoD</u> - <u>Lolekaa silty clay, 15 to 25 percent slopes - This soil is on side slopes of terraces and along drainageways. Runoff is medium, and the erosion hazard is moderate. This soil type is found in one area along the shoulder of Kaipapa'u Stream.</u>

The Waikane series consists of well-drained soils on the island of O'ahu. These soils developed in alluvium and colluvium derived from igneous rock.

<u>WpB - Waikane silty clay, 3 to 8 percent slopes - Runoff is slow and erosion hazard is slight.</u>

<u>WpC</u> - Waikane silty clay, 8 to 15 percent slopes - On this soil, runoff is slow to medium and the erosion hazard is slight to moderate.

No areas of contaminated soil are expected to be encountered in the area.

<i>C.6</i> -	Nature	and S	equence	of	Construction .	Activity

What is the function of the construction activity (Please check all applicable activity(ies))?									
Residential	Commercial	$oldsymbol{\Box}$ Industrial		Linear Utility					
☐ Other (please	e specify):		<u> </u>						

What is being constructed? The existing Kaipapa'u Stream Bridge is deficient due to age and dilapidation, and requires demolition and replacement. The project area required for construction would be approximately 1.6 acres. The project's scope of work includes installation of erosion controls, clearing, grubbing, grading, temporary placement of sand bags to redirect the stream during construction relocation and installation of waterlines and electrical lines, construction and use of a temporary detour roadway and Acrow bridge, demolition of the existing bridge and construction of a new bridge, partial demolition and reconstruction of the abutments, removal of the existing center pier wall, excavation & construction of eight new drilled shafts outside the stream channel, maintenance dredging, and bank stabilization with shotcrete and dumped rip-rap. All excavated material (soils & dewatering effluent) will be placed in a temporary retention area for treatment and disposal. No excavated material will discharge to the stream.

The replacement of the Kaipapa'u Stream Bridge and maintenance work will be completed through phased construction and demolition. Silt fences will be installed on down slope portions of the project site. A staging area, temporary dewatering basin, temporary concrete wash-out basin, and stabilized construction entrances will be prepared.

Sandbags will be used to divert normal-stream flow around the work area. The temporary placement of sandbags to redirect the stream during construction of the temporary detour road (sandbag diversion approximately 610 feet long) and new bridge (sandbag diversion approximately 600 feet long) and will be designed based on the Contractor's means and methods. It is assumed that 7 sandbags (1-foot-wide each) will be placed at the base (4 sandbags on the side of the channel closer to the work area, and 3 sandbags on the other side of the temporary channel). Placement of the temporary sandbag diversion will require approximately 25 cubic yards (CY) of temporary fill placed within the Mean Higher High Water (MHHW) and 5 CY of temporary fill placed within the Ordinary High Water Mark (OHWM).

A temporary construction entrance ramp will be constructed on the mauka and makai portions of the stream comprised of dumped rip-rap. There will be no interruption of stream flow. In-stream work will be completed during the low rainfall season (August to October), and during fair weather conditions.

Approximately 270 CY of maintenance dredging will be performed to remove accumulated sediment and debris from under and around the bridge partially within the MHHW. Approximately 5 CY is located within the MHHW of Kaipapa'u Stream. The excavated spoils and demolition debris will not be discharged into the stream. Spoils will be dewatered in a detention basin and dried debris will be disposed of off-site at a County-approved landfill. Removed material will be contained in a temporary stockpile site with implemented BMPs to contain and prevent material from comingling with storm water runoff and entering into State waters. A solid waste disclosure form will be submitted to the Department of Health (DOH) Solid Waste Branch.

The temporary Acrow bridge will be 90 feet long by 42 feet wide, or approximately 3,780 square feet, and constructed with pre-cast concrete pier columns supporting the steel deck. The bridge will be comprised of two lanes and a pedestrian walkway on the makai side of the Kaipapa'u Stream Bridge to mitigate traffic impacts during construction. The Acrow bridge will be constructed and installed in two 45-foot spans and supported by five pre-cast concrete piers, one of which is located within the MHHW. Placement of the one pier in the MHHW will require 1 CY of temporary fill below the MHHW. Temporary dumped rip-rap will be placed around the Acrow bridge pier within the MHHW and be sized approximately 54 feet long by 15 feet wide by 2 feet deep, or 810 square feet, with a volume of 50 CY. A 6-foot temporary layer of filter rock will be placed under the rip-rap with a volume of approximately 13 CY. Upon completion of the bridge replacement, the Acrow bridge and piers will be removed and disturbed areas restored to their pre-construction condition.

Demolition of the existing Kaipapa'u Stream Bridge will include the removal of the existing concrete center pier wall, of which approximately 5 CY is located within the MHHW(26 feet long by 4 feet wide or approximately 104 square feet).

The new replacement bridge will be 110 feet long by 57 feet wide, or approximately 6,270 square feet, and include two 12-foot travel lanes plus two 8.5-foot shoulders, two 5-foot pedestrian walkways/bicycle lanes, reinforced guardrails, and drainage features. The new bridge will be constructed using prestressed concrete planks and cast-in-place bridge decks. The new right-of-way (ROW) will be 66 feet wide. The project will involve partial demolition and reconstruction of the abutments requiring excavation and construction of eight new 4-foot drilled

shafts outside of the OHWM and MHHW. All work proposed for the reconstruction of the Kaipapa'u Stream Bridge would be completed above and along the outer banks of the streams and no work is proposed within the stream. The new bridge would accommodate utilities currently attached to the existing bridge. No debris would be allowed to fall into or enter the stream.

The north bank makai of the bridge will be stabilized with dumped rip-rap outside of the MHHW. In addition to stabilization, the dumped rip-rap will provide construction access to the stream for mechanical equipment.

A section of the existing wall running along the northern bank mauka of the bridge collapsed during a major storm in 2008. Emergency repairs were conducted to create a wall of sandbags. The existing sandbag wall, located outside the OHWM, will be stabilized with the placement of basalt boulders at the toe of the sandbags. The existing sandbags will then be covered with shotcrete. Work for the stabilization of the wall will be performed above the OHWM. No debris would be allowed to fall into or enter the stream.

Portions of an existing 12-inch diameter waterline beneath Kaipapa'u Stream will be repaired. The portions of the 12-inch waterline to be replaced are located outside the stream (see Attachment B, Construction Drawings, C-20, C-28) and will be repaired via open trench (approximately 85 linear feet). The existing 12-inch waterline under the stream will be temporarily removed from service during the repairs and then reconnected and placed back into service following completion of the 12-inch waterline work. During repairs a temporary 12-inch 125-foot-long or 125 square foot waterline will be placed on the existing pedestrian bridge.

The replacement of an existing 16-inch diameter will require the removal of the existing waterline, placement of a temporary waterline, and installation of the new 16-inch diameter waterline over the stream. The temporary 16-inch diameter 250-foot-long or 333 square foot waterline will be placed on the temporary detour bridge during construction. The new permanent 16-inch diameter 155 feet long or 207 square feet waterline will be installed over the stream within the new bridge 3.2-foot-wide concrete bridge encasement. Following the installation of the 16-inch permanent waterline the temporary waterline will be removed.

Above the MHHW and OHWM, the project will also include the reconstruction of the 6-foothigh concrete wall with wood fence panels on the northern side of the bridge, replacement of fencing, acquisition of two properties (Tax Map Keys (TMKs) 5-4-18: 3 and 5-4-11: 20), removal of an existing septic system and leaching field on TMK: 5-4-11: 20, and demolition of two buildings on TMK 5-4-18: 3 and one building on TMK 5-4-11: 20. Acquisition of property

and demolition of structures is required for construction access and for the installation of waterlines to be supported on the outside edges of the new bridge.

In-water work would only be required for the minor maintenance dredging, removal of the existing bridge center pier wall, temporary placement of sandbags to divert the steam around the open work area, and temporary placement of one Acrow bridge pier within Kaipapa'u Stream.

Describe the scope of work and major construction activities you wish to be covered in this NPDES application, including baseyards and staging areas. You may only include project areas where the locations of impervious structures are known; project areas where the final grades are known; and work areas that will be performed by one (1) general contractor. A separate NPDES application will be required for all other project areas.

The existing 74-year-old Kaipapa'u Stream bridge has been evaluated by HDOT as being structurally deficient and presently does not meet design standards. This proposed bridge project is needed to mitigate bridge maintenance concerns, increase traffic safety (for motorists and pedestrians) and meet the projected vehicle usage of the Kamehameha Highway. This project is one in a series of bridge replacements being implemented by the State Department of Transportation and Federal Highway Administration on O'ahu.

The project's scope of work includes installation of erosion controls, clearing, grubbing, grading, temporary placement of sand bags to redirect the stream during construction relocation and installation of waterlines and electrical lines, construction and use of a temporary detour roadway and Acrow bridge, demolition of the existing bridge and construction of a new bridge, partial demolition and reconstruction of the abutments, removal of the existing center pier wall, excavation & construction of eight new drilled shafts outside the stream channel, maintenance dredging, and bank stabilization with shotcrete and dumped rip-rap. All excavated material (soils & dewatering effluent) will be placed in a temporary retention area for treatment and disposal. No excavated material will discharge to the stream.

The replacement bridge will measure approximately 110 feet long by 57 feet wide and will meet State and Federal roadway, bridge, and seismic standards. The design includes two 12-foot travel lanes plus two 8.5-foot shoulders, two 5-foot pedestrian walkways/bike lanes, reinforced guardrails, and drainage features. The approach and trailing guardrails will comply with the current standards of the HDOT. Shotcrete and dumped rip-rap will be installed on the banks of the stream to stabilize the embankment. The proposed bridge, temporary Acrow bridge, and approach roads shall conform to AASHTO and HDOT design criteria for roadway widths and safety features.

The sequencing of construction activity is as follows:

- Install best management practices (BMPs)/erosion control measures (see **Sheet C-18**).
- Install temporary 12" water line and relocate existing 12" water line (see **Sheets C-20**, **C-28**, and **C-29**).
- Relocate electrical utilities.
- Construct trial and load test drilled shafts and perform load test.
- Construct detour roadway and temporary Acrow bridge.
- Demolish existing Kaipapa'u Stream Bridge. Expose existing 16" water line jacket and concrete support system.
- Construct Phase 1 of new Kaipapa'u Stream Bridge (see **Sheets S0.7, S0.7A, S0.7B**).
- Partially remove detour roadway and temporary bridge. Construct temporary pavement transitions, signing and pavement markings.
- Construct Phase 2 of new Kaipapa'u Stream Bridge (see **Sheets S0.8, S0.8A, S0.8B**).
- Remove remainder of detour roadway and temporary bridge.
- Construct sand bags and shotcrete lining along north bank, upstream of Kaipapa'u Stream Bridge (see **Sheet C-18**).
- Construct dumped riprap along north and south bank, downstream of Kaipapa'u Stream Bridge (see **Sheets C-16** and **C-18**).
- Construct AC pavement (see **Sheet C-16**).
- Construct final signing and pavement markings.
- Remove temporary BMPs.

On-site staging areas will be used as designated areas where vehicles, supplies and construction equipment are positioned for access and use during the construction process. The locations of the staging and storage areas may be changed by the Contractor depending on his construction means and methods. Equipment may include, but is not limited to: bulldozers, excavators, drilling rig, loaders, grader, compaction rollers, backhoe, cranes, trucks delivering supplies, pneumatic hand-operated tools, dewatering pumps, asphaltic rock products and fill material, and related construction materials which will include the following: Concrete and shotcrete, asphaltic Concrete, precast structures, pipes, paints (enamel and latex), cleaning solvents, rebar, wood, tar, masonry block, steel sheet piles, rocks/boulders, sandbags, soil fill material, and acrow steel bridge deck.

C.7 - Existing or Pending Permits, Licenses, or Approvals

Place a check next to all applicable Federal, State, or County permits, Licenses, or approvals for the project and specify the permit number.

☑ Other NPDES Permit or NGPC File No.: NPDES Forms F (Hydrotesting Activities) and G (Dewatering Activities)

☑ Department of the Army Permit (Section 404): POH-2005-00342 (April 4, 2019)

If your project requires work in, above, under or adjacent to State waters, please contact the
Army Corps of Engineers (COE) Regulatory Branch at (808) 438-9258 regarding their
permitting requirements. Provide a copy of the COE permitting jurisdictional determination
(JD) or the JD with COE Person's Name, Phone Number, and Date Contacted.
☐ Facility on SARA 313 List (identify SARA 313 chemicals on project site:
☐ RCRA Permit (Hazardous Wastes):
☐ Section 401 Water Quality Certification: The project is exempted from obtaining a Section
401 Water Quality Certification (WQC), as provided by Senate Bill 1016 SD1 HD1 (expires
June 30, 2022).
☑ Other (Specify): Special Management Permit (Resolution 278-CD1); U. S. Coast Guard
Clearance (obtained); Section 106, National Historic Preservation Act, Consultation (completed);
Section 7, Endangered Species Act, Consultation (completed); Section 4(f) Department of
Transportation Act, Consultation (completed); Stream Channel Alteration Permit (exempt per
Senate Bill 1016 SD1 HD1); HDOT Plan Review (pending); Grading Permit (pending); Coastal
Zone Management Federal Consistency Review (pending)
County-approved Erosion and Sediment Control Plan and/or Grading Permit
a. Is a County-approved Erosion and Sediment Control Plan and/or Grading Permit, where
applicable for the activity and schedule for implementing each control, required?
✓ Yes. Please complete Section C.7.b below and skip Section C.7.c.
☐ No. Please complete Section C.7.c below and skip Section C.7.b.
b. Is a copy County-approved Erosion and Sediment Control Plan and/or Grading Permit,
as appropriate for the activity and schedule for implementing each control, attached?
☐ Yes, see Attachment
☑ No, the County-approved Erosion and Sediment Control Plan and/or Grading Permit,
as appropriate for the activity and schedule for implementing each control, will be
submitted at least 30 calendar days before the start of construction activities.
c. Please select and complete at least one (1) of the following items to demonstrate that a
County-approved Erosion and Sediment Control Plan and/or Grading Permit, as
appropriate for the activity and schedule for implementing each control, is not required.
\square See Attachment for the County written determination.
☐ Provide the County contact person information (Name, Department, Phone Number, and Date Contacted):
and Date Contacted):
Other (specify):

C.8 - Project Site Maps and Construction Plans/Drawings

Attach, title, and identify all maps (pdf - minimum 300 dpi) listed below, in Attachment A. Please reference which maps account for the features listed below.

- a. Island on which the project is located. O'ahu. See Attachment A-1, Figure 1, Project Location
- b. Vicinity of the project on the island. See Attachment A-1, Figure 1, Project Location
- c. Legal boundaries of the project. See Attachment A-1, Figure 2, Boundary Map
- d. Receiving State water(s) from Section 6 of e-Permitting form and receiving separate drainage system(s) from Section 7 of e-Permitting form, identified and labeled.

See Attachment A-1, Figure 4, Discharge Points

- e. Location of ALL discharge points from Section 6 of e-Permitting form with identification numbers. See Attachment A-1, Figure 4, Discharge Points
- f. Boundaries of 100-Year flood plans. See Attachment A-1, Figure 5, Flood Zones
- g. Areas of soil disturbance. See Attachment A-2, Construction Plans/Drawings
- h. Location(s) of impervious structures (including buildings, roads, parking lots, etc.) after construction is completed. See Attachment A-2, Construction Plans/Drawings
- i. Pre-Construction Topography including approximate slopes and drainage patterns for the entire Facility/Project site to the receiving storm water drainage system (if applicable) or to the receiving State water(s) (with flow arrows). See Attachment A-2, Construction Plans/Drawings
- j. During-Construction Topography (after major grading activities) including approximate slopes and drainage patterns for the entire Facility/Project site to the receiving storm water drainage system (if applicable) or to the receiving State water(s) (with flow arrows).

 See Attachment A-2, Construction Plans/Drawings
- k. Post-Construction Topography including approximate slopes and drainage patterns for the entire Facility/Project site to the receiving storm water drainage system (if applicable) or to the receiving State water(s) (with flow arrows). See Attachment A-2, Construction Plans/Drawings

C.9 - Flow Chart or Line Drawing

Attach or insert in Attachment A, a flow chart showing the following (Check each item, as applicable):

See Attachment A-4, Storm Water Flow Chart

- \square a. Storm water entering the project from off-site areas
- ☑ b. General route taken by storm water through the project (show the routes through different drainage areas)
- ☑ c. Treatment system(s) utilized for the reduction of sediment (e.g., silt fence, earth berm, detention basin, vegetated swale, etc.)

- ☑ d. Best Management Practices (BMPs) utilized to prevent erosion (e.g., erosion control mats, reduced open area, revegetation, etc.)
- ☑ e. Quantity of flow through each applicable route from upslope to the receiving State water
- ☐ f. Drainage system(s) receiving storm water from the project, as applicable (e.g., City and County of Honolulu Municipal Separate Storm Sewer System (MS4), etc.)
- \square g. State water name(s) receiving storm water from the project

Indicate which item(s) are not identified and explain why the item(s) are not identified No storm water is expected to enter the project from off-site or adjacent areas.

C.10 - Construction Schedule

Provide the following estimated dates:

The date when construction activity will begin The estimated scheduled start time for construction is January 2021. The overall duration of the project is expected to be approximately three years. A detailed schedule of construction activity will be completed when a contractor is selected for the project and provided to DOH-CWB 30 days prior to the start of construction.

The date when each major construction activity begins Same as above.

The date when the Notice of Cessation form will be submitted Same as above.

C.11 – Storm Water Pollution Prevention Plan (SWPPP)

Include your SWPPP that complies with HAR, Chapter 11-55, Appendix C in Attachment A.

You are responsible for the design, implementation, operation, and maintenance of the SWPPP to ensure that storm water discharges associated with construction activities will not cause or contribute to a violation of HAR, Chapter 11-54, Chapter 11-55, and Chapter 11-55 Appendix C.

The contractor may augment or improve BMPs for discharges of storm water associated with construction activity after the NPDES permit is issued in accordance to HAR, Chapter 11-55, Appendix C. These amendments do not have to be submitted to the DOH-CWB, but shall be kept on-site and available upon request.

See Attachment A-5, Storm Water Pollution Prevention Plan (SWPPP) and In-Water Pollution Prevention Plan (IWPPP).

Attachments

Attachment A - Project Site Maps, Construction Plans/Drawings, Flow Chart, and SWPPP (Sections C.8, C.9, & C.11)

PROJECT SITE MAPS, CONSTRUCTION PLANS/DRAWINGS, FLOW CHART, AND SWPPP

Contents

Attachment A – 1: Project Site Maps

Attachment A-2: Construction Plans/Drawings

Attachment A-3: Quantity of Strom Water Discharge Calculations

Attachment A - 4: Storm Water Flow Chart

Attachment A – 5: Storm Water Pollution Prevention Plan (SWPPP) and In-Water Pollution Prevention Plan (IWPPP)

Attachment A – 1: Project Site Maps

Figure 1, Project Location

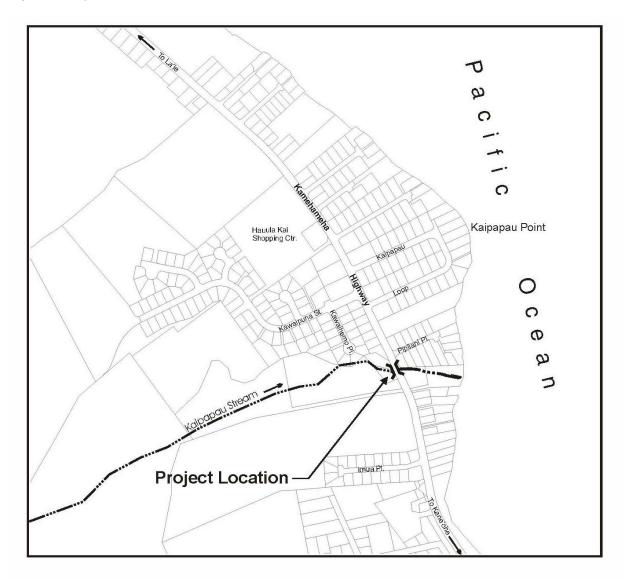
Figure 2, Boundary Map

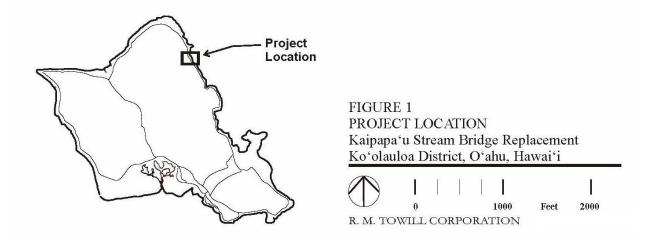
Figure 3. Soil Classifications

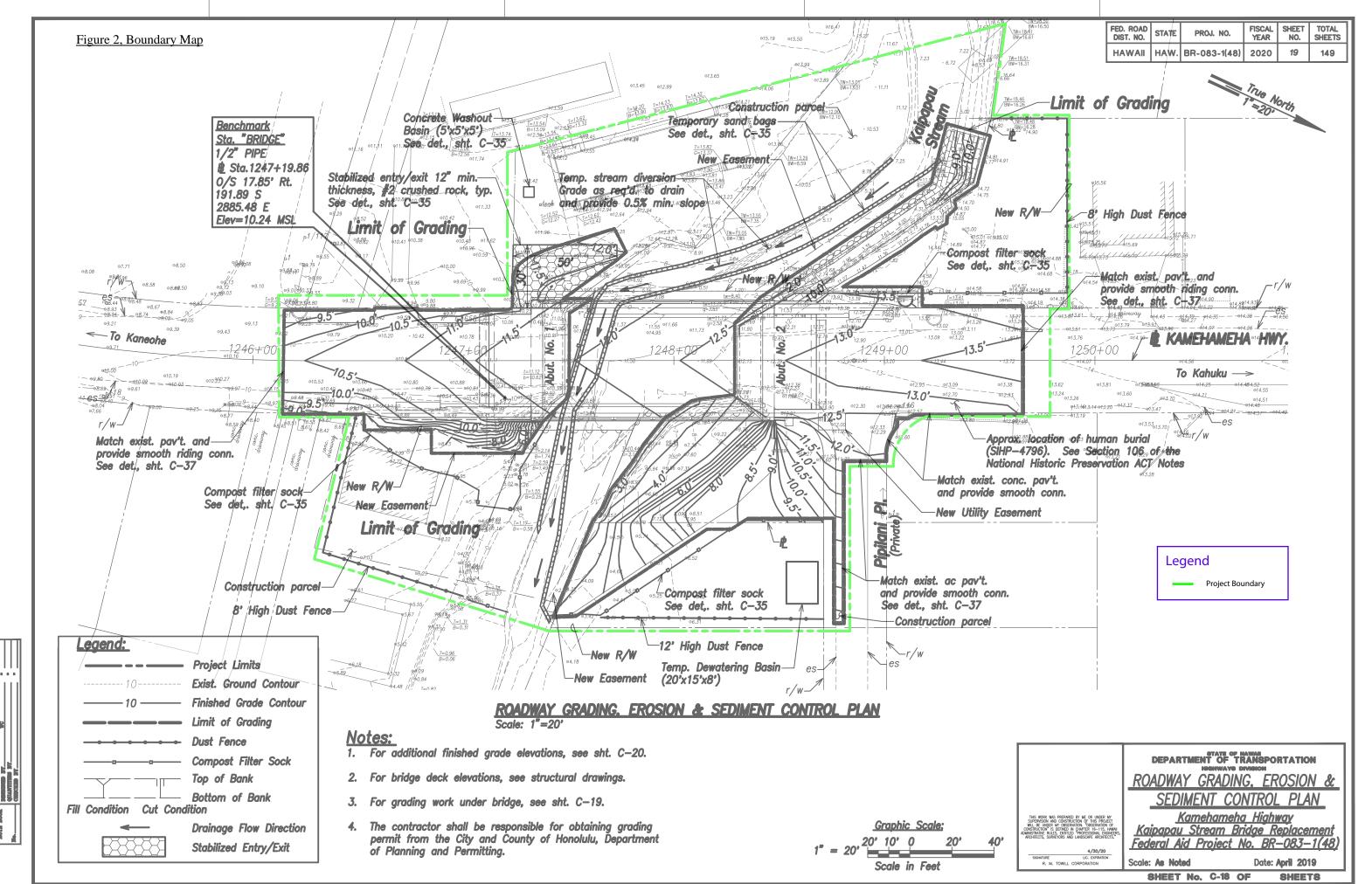
Figure 4. Discharge Locations

Figure 5. Flood Zones

Figure 1, Project Location

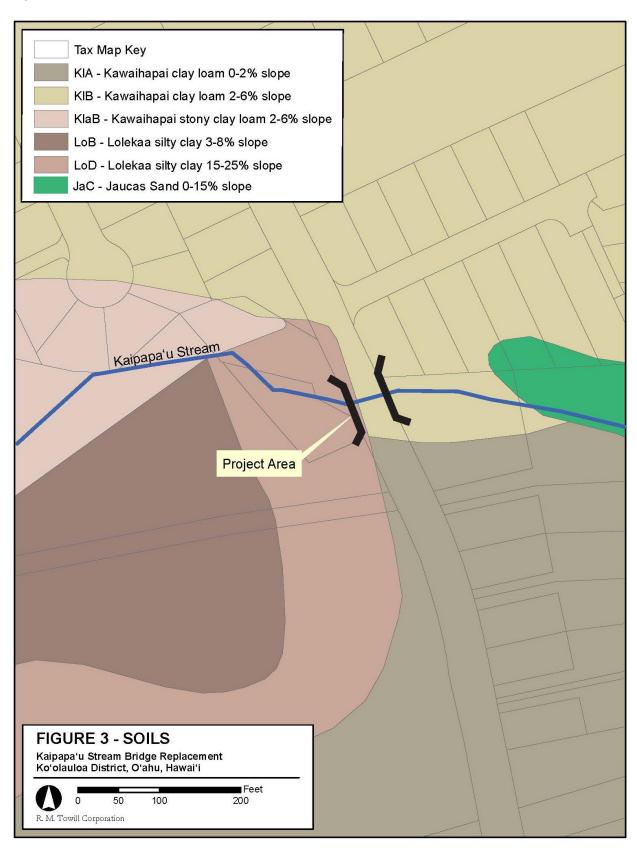






RMTC JOB NO.: 1-19548-0E

Figure 3. Soil Classifications



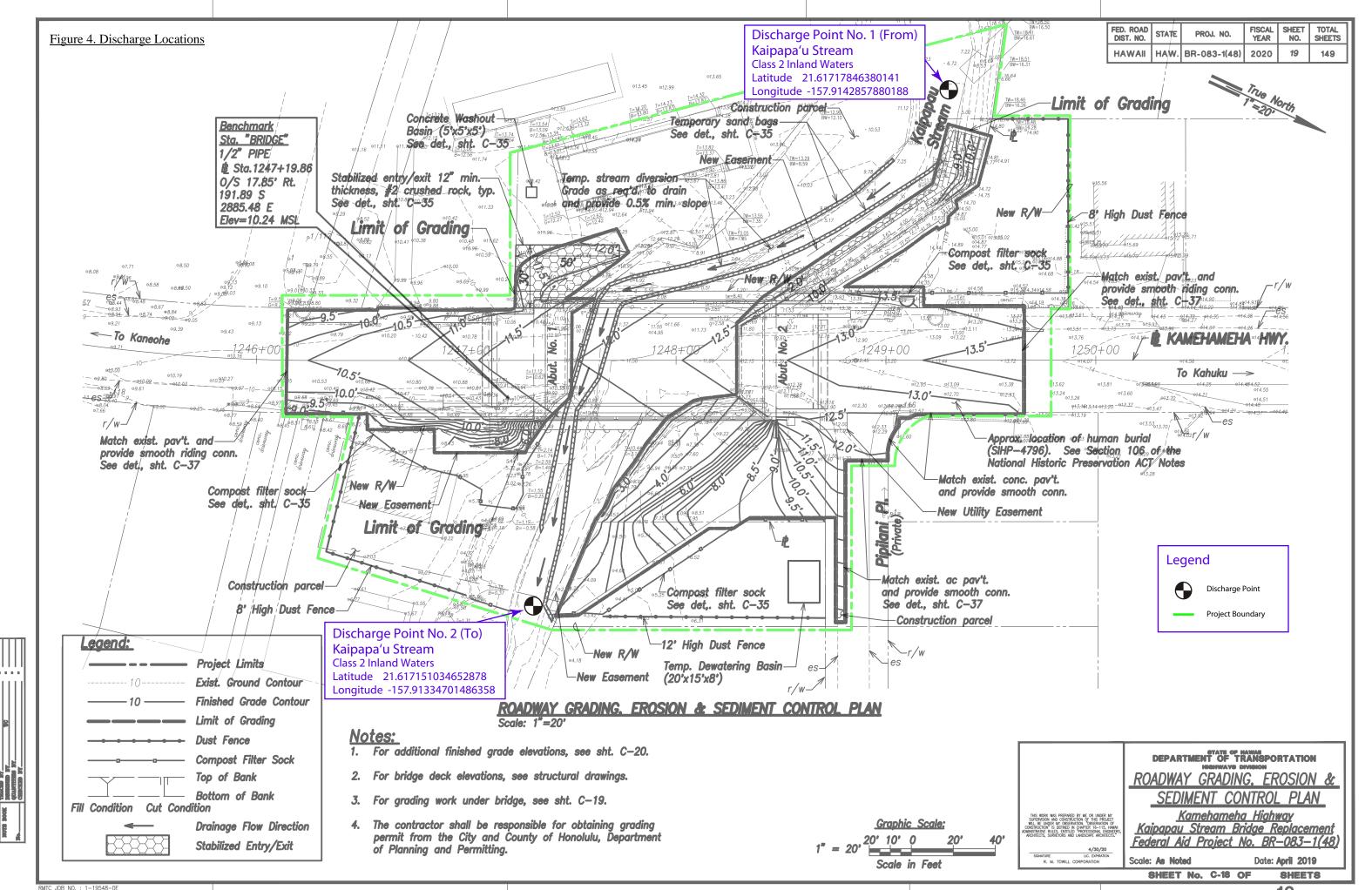
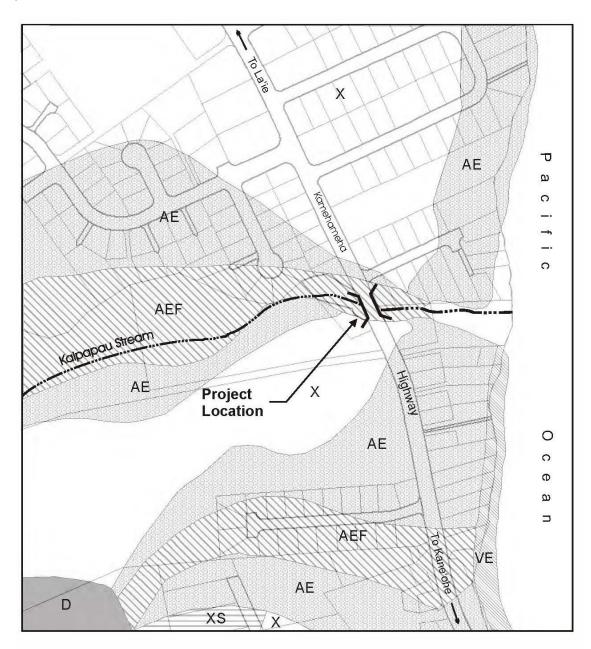
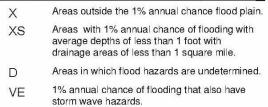


Figure 5. Flood Zones



LEGEND



AE Areas with 1% annual chance of flooding.

AEF

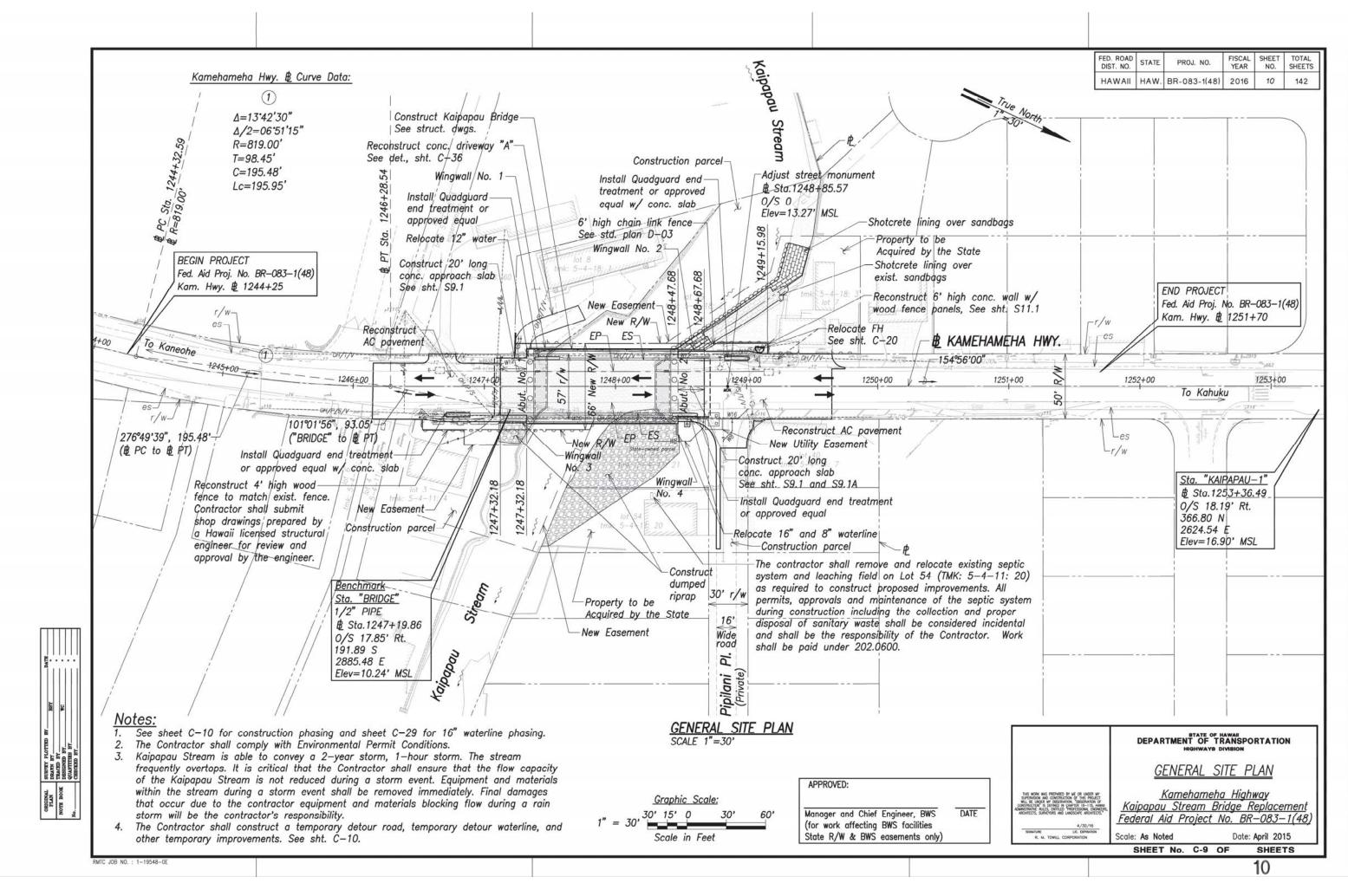
The watercourse or portion of the flood plain which must be reserved in order to carry or discharge the regulatory flood without cumulatively increasing the flood elevation of the flood plain more than a foot at any given point.

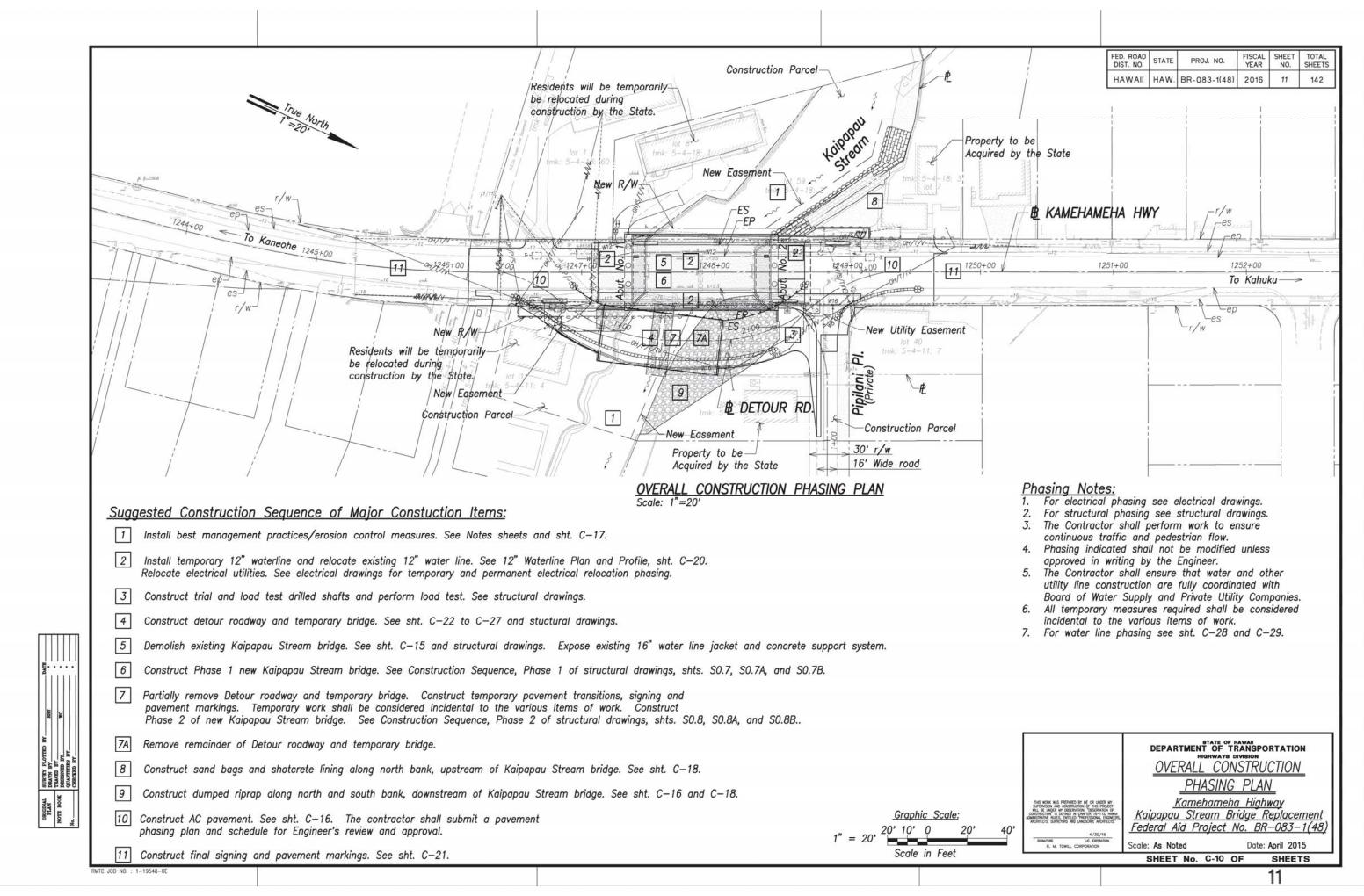
FIGURE 5 FLOOD ZONE MAP Kaipapa'u Stream Bridge Replacement Ko'olauloa District, O'ahu, Hawai'i

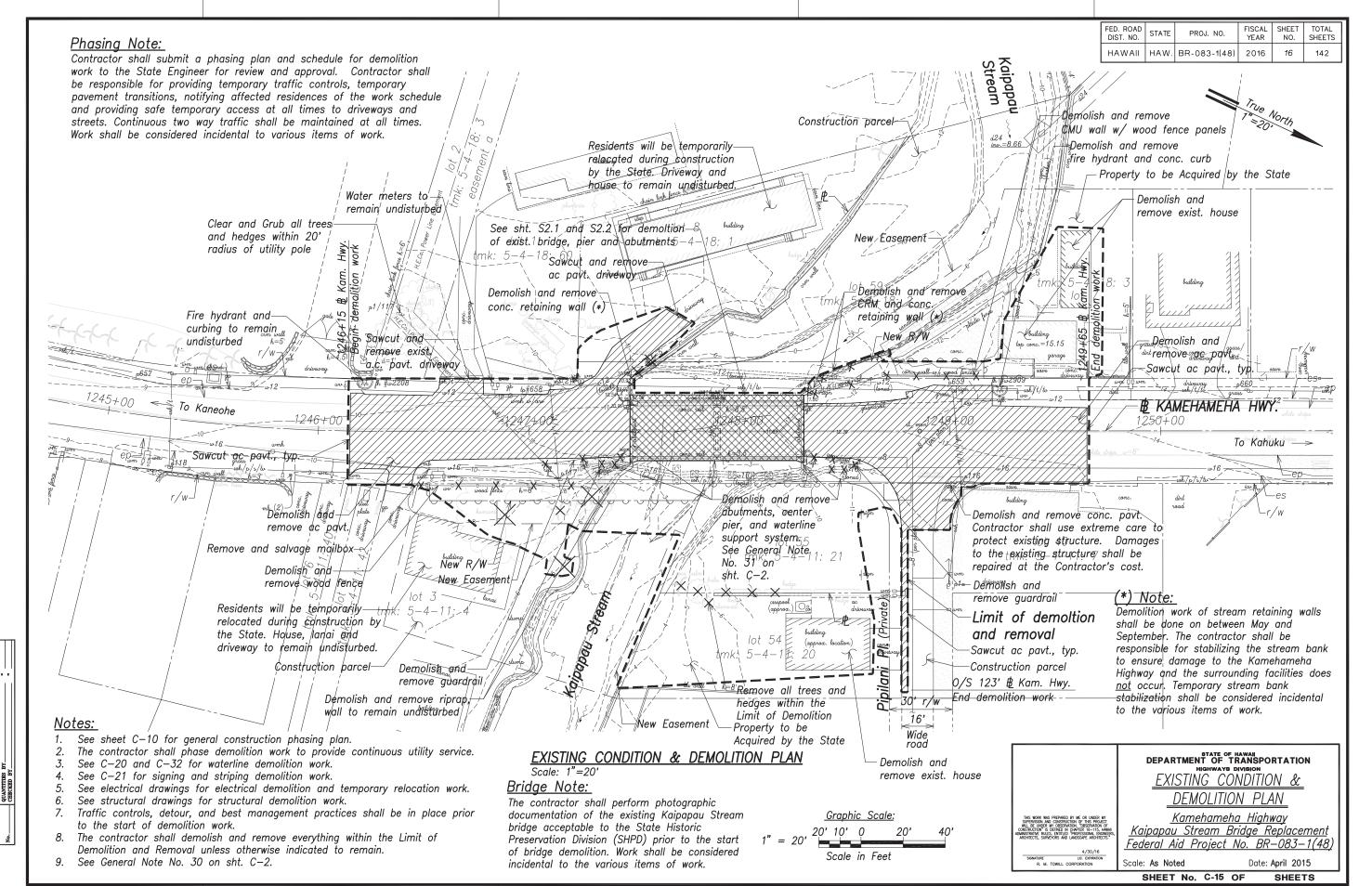


R. M. TOWILL CORPORATION

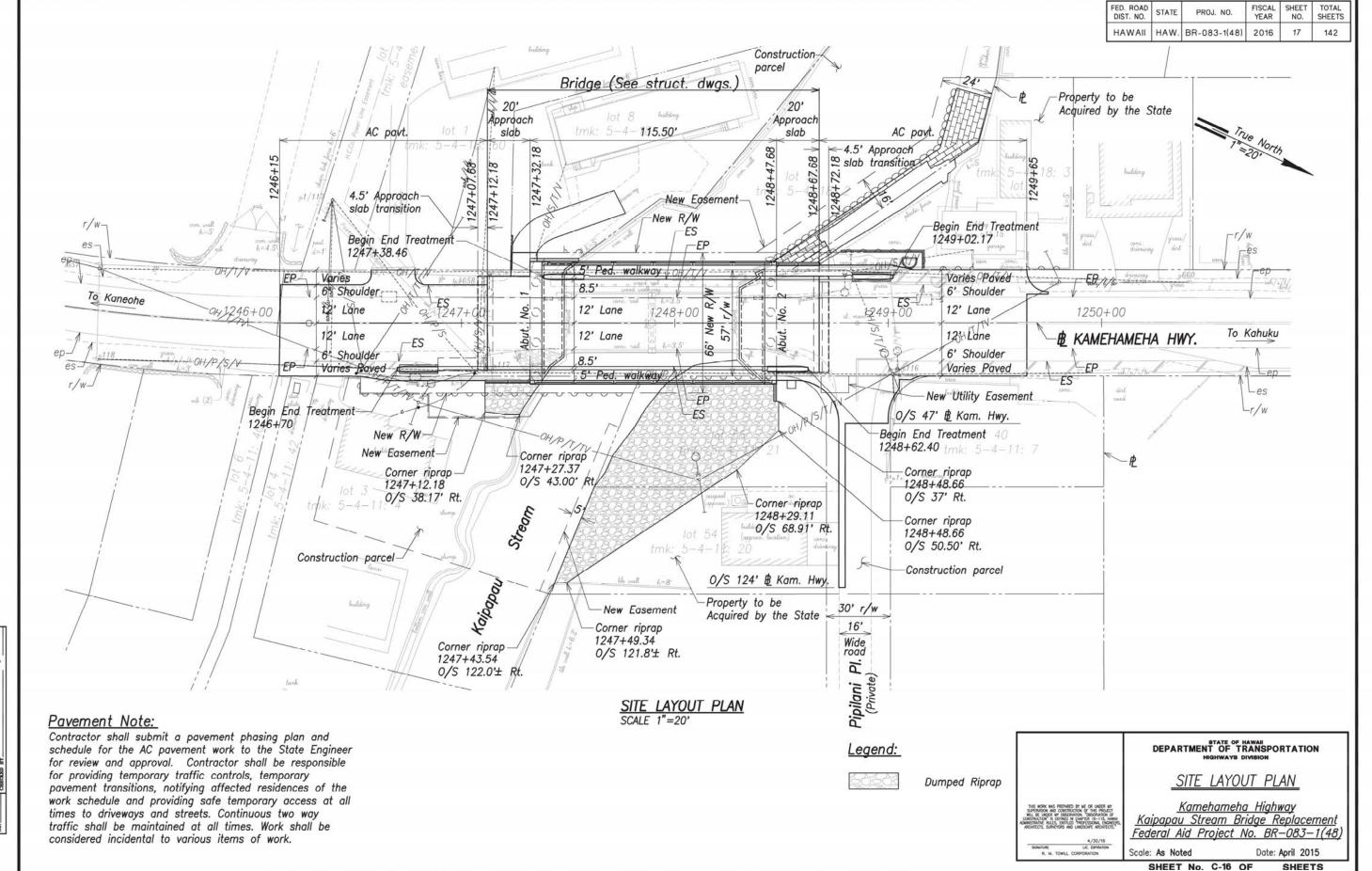
Attachment A-2: Construction Plans/Drawings





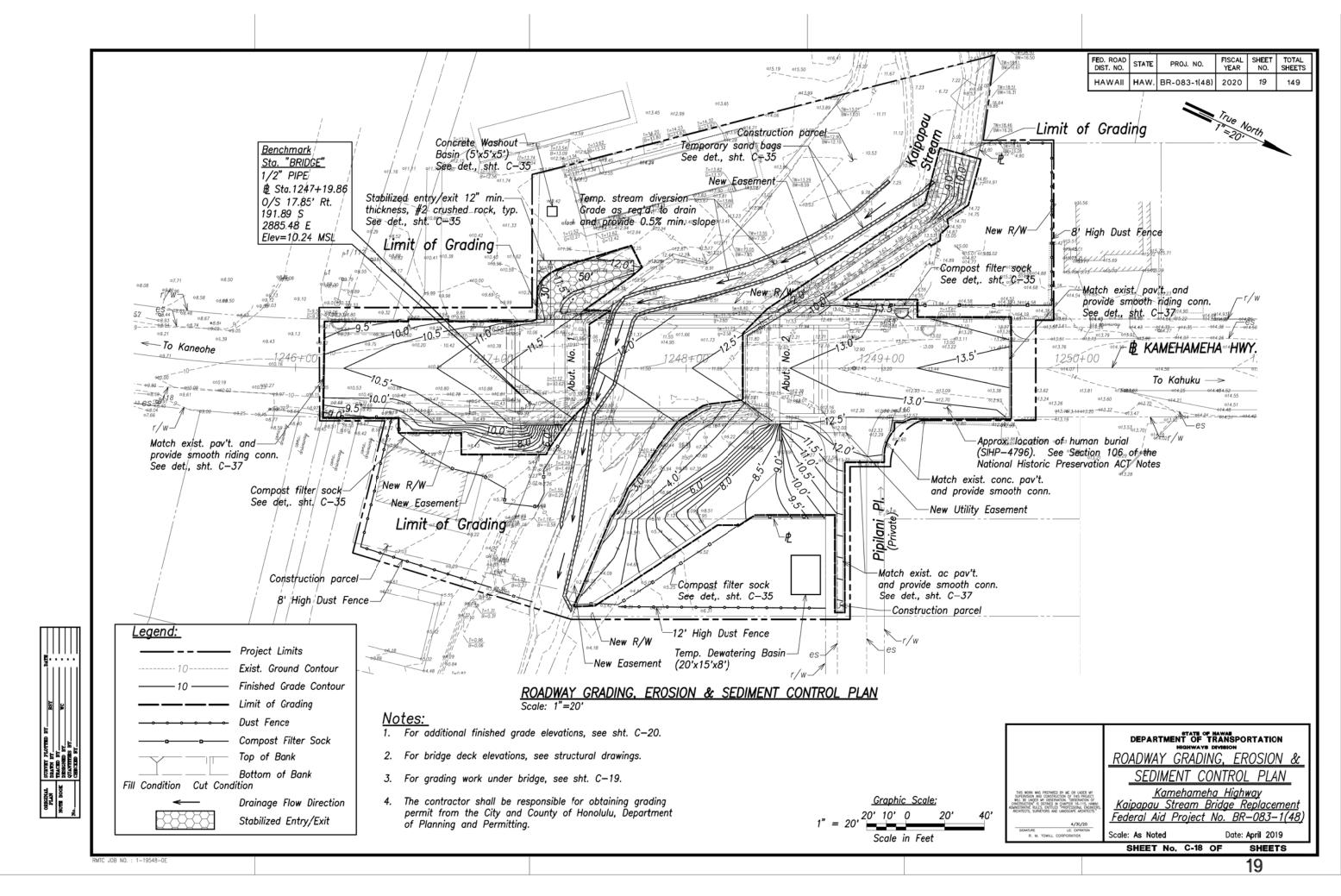


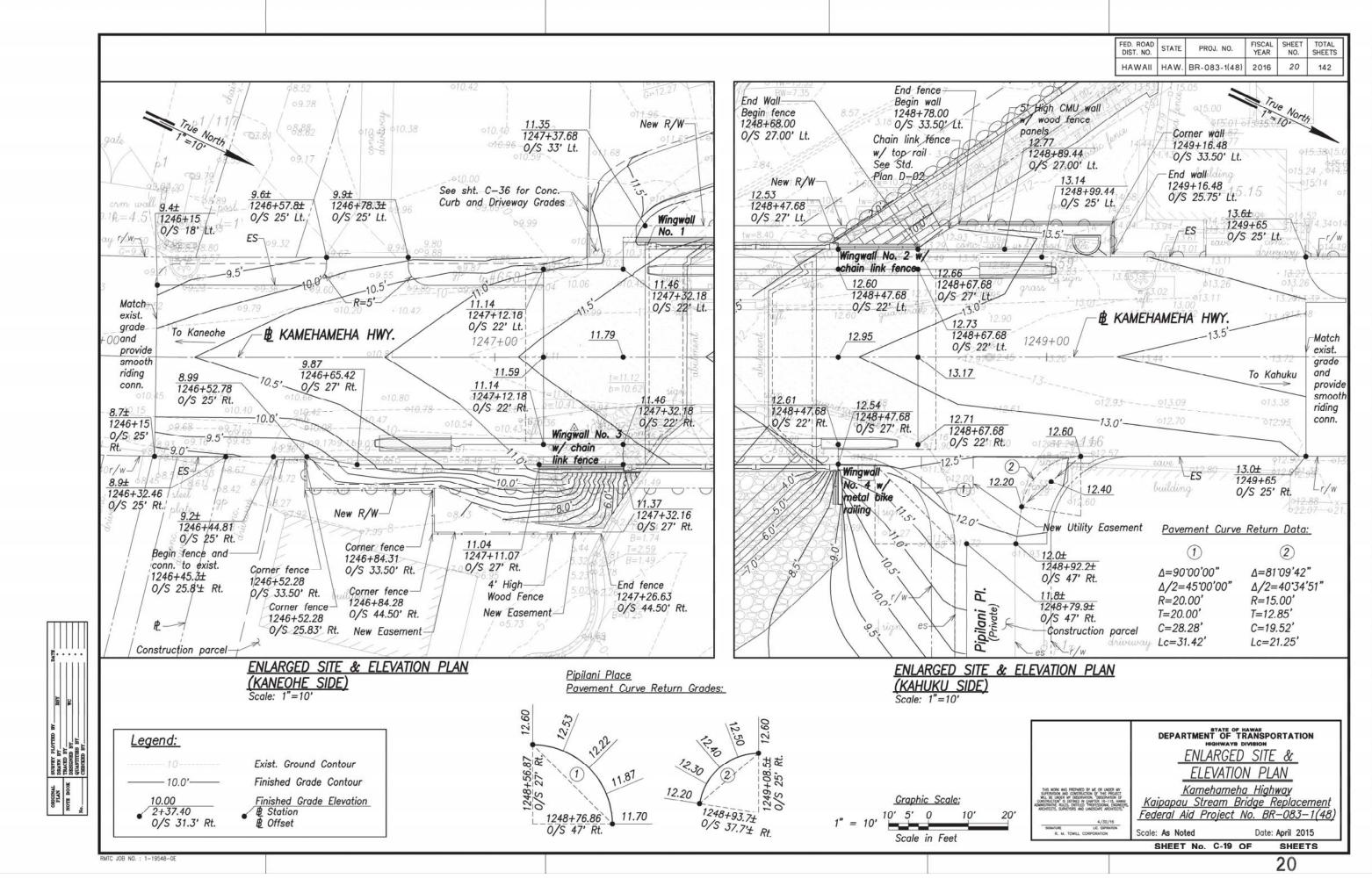
RMTC JOB NO. : 1-19548-0E

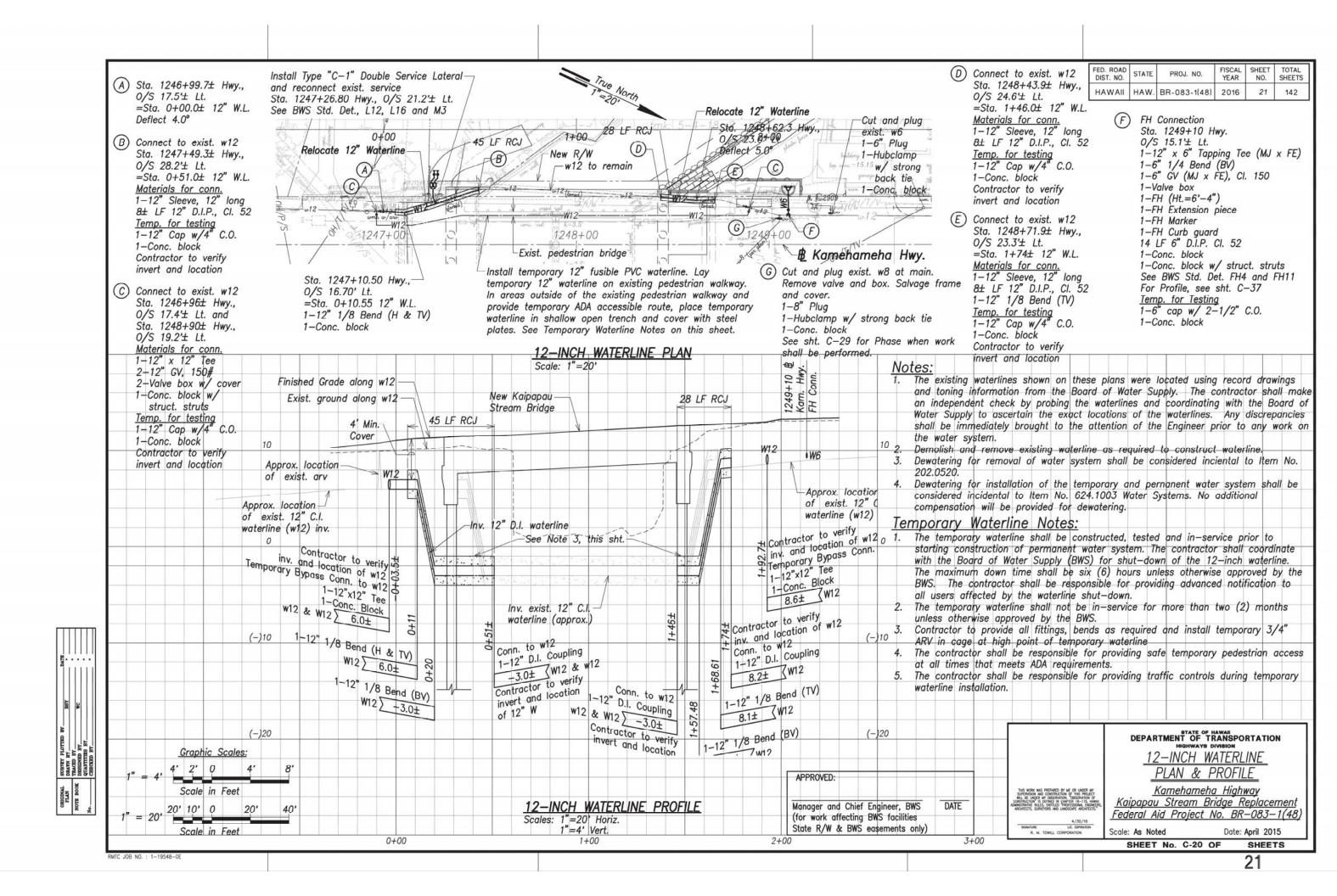


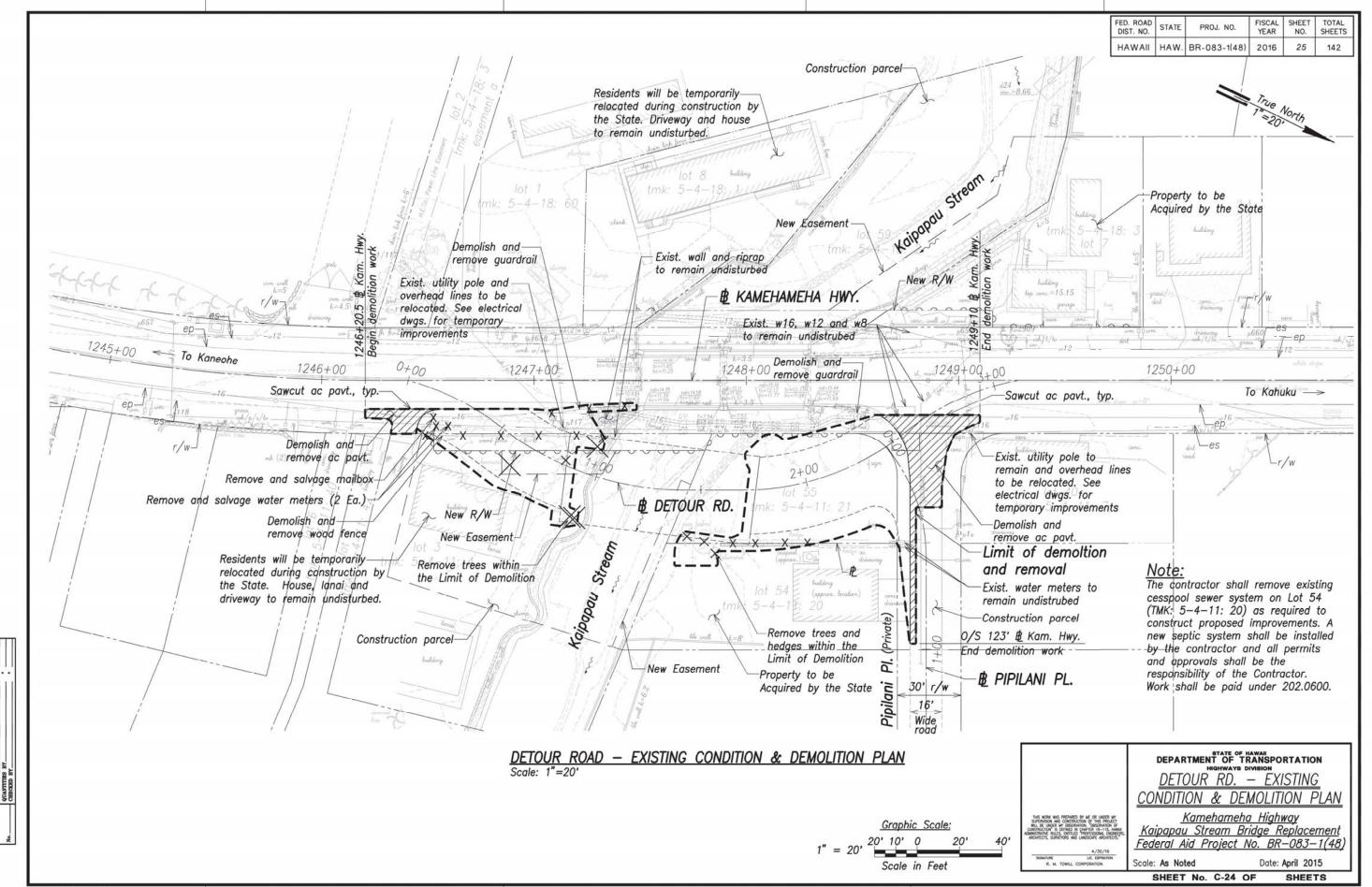
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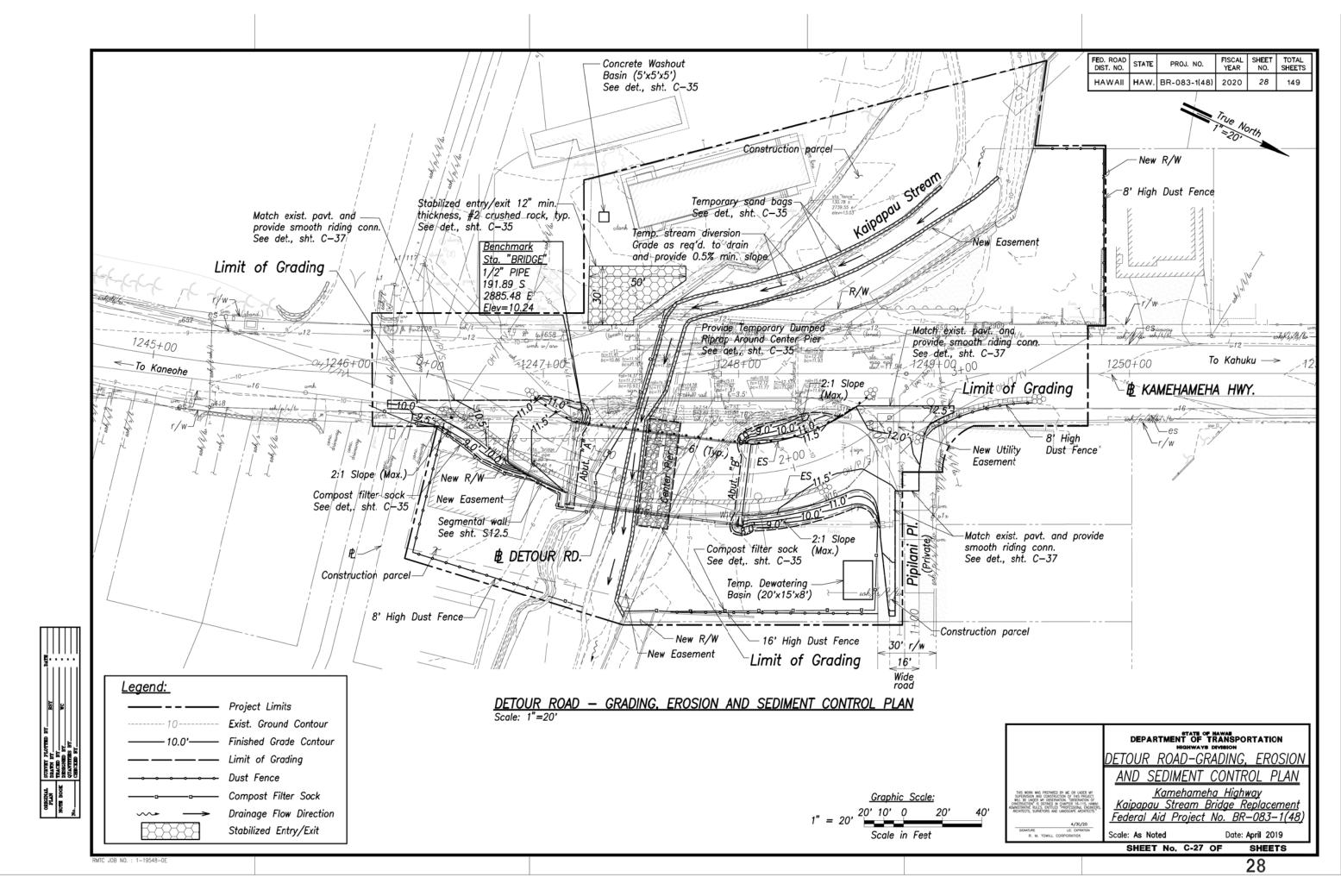
RMTC JOB NO. : 1-19548-0E

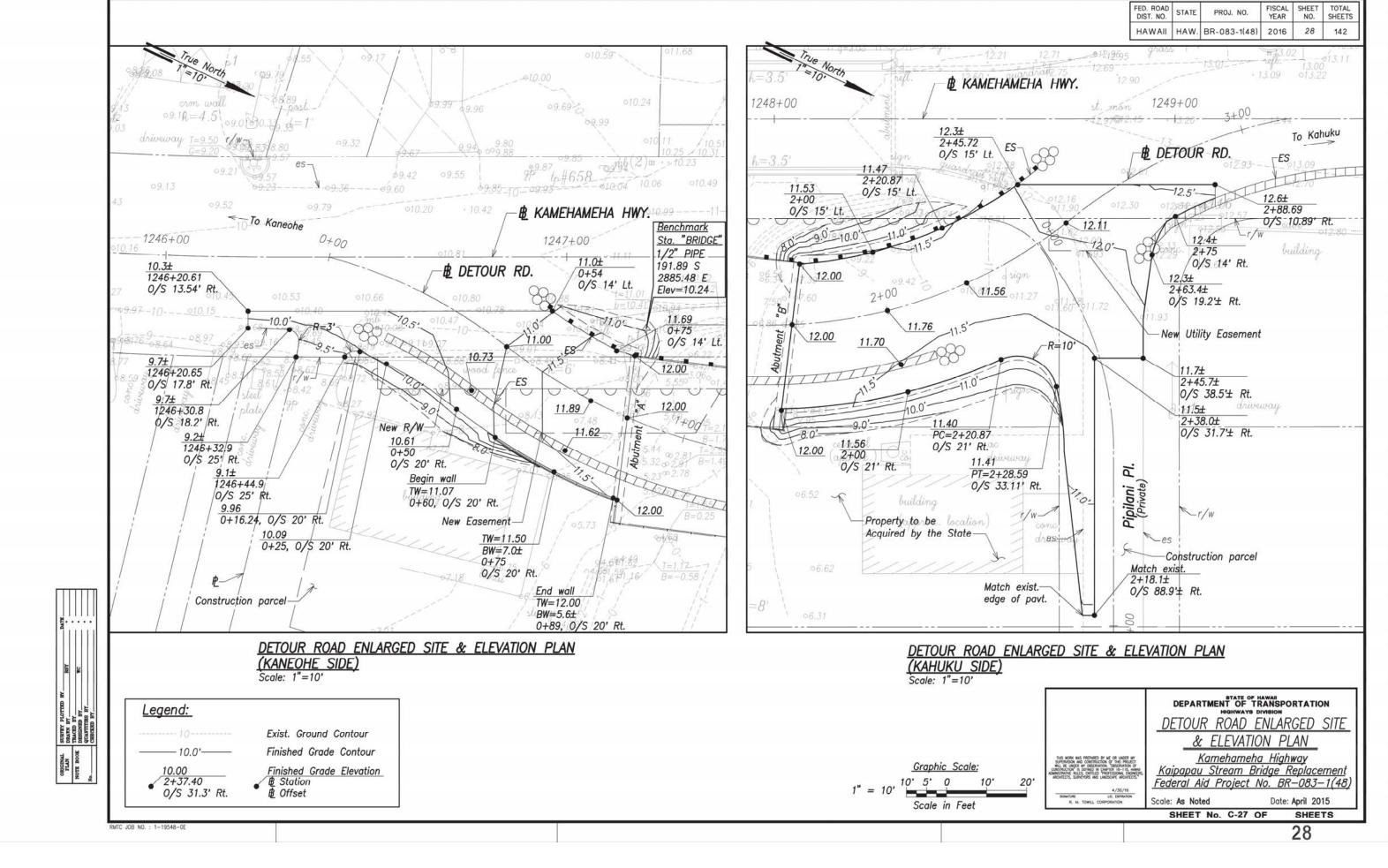


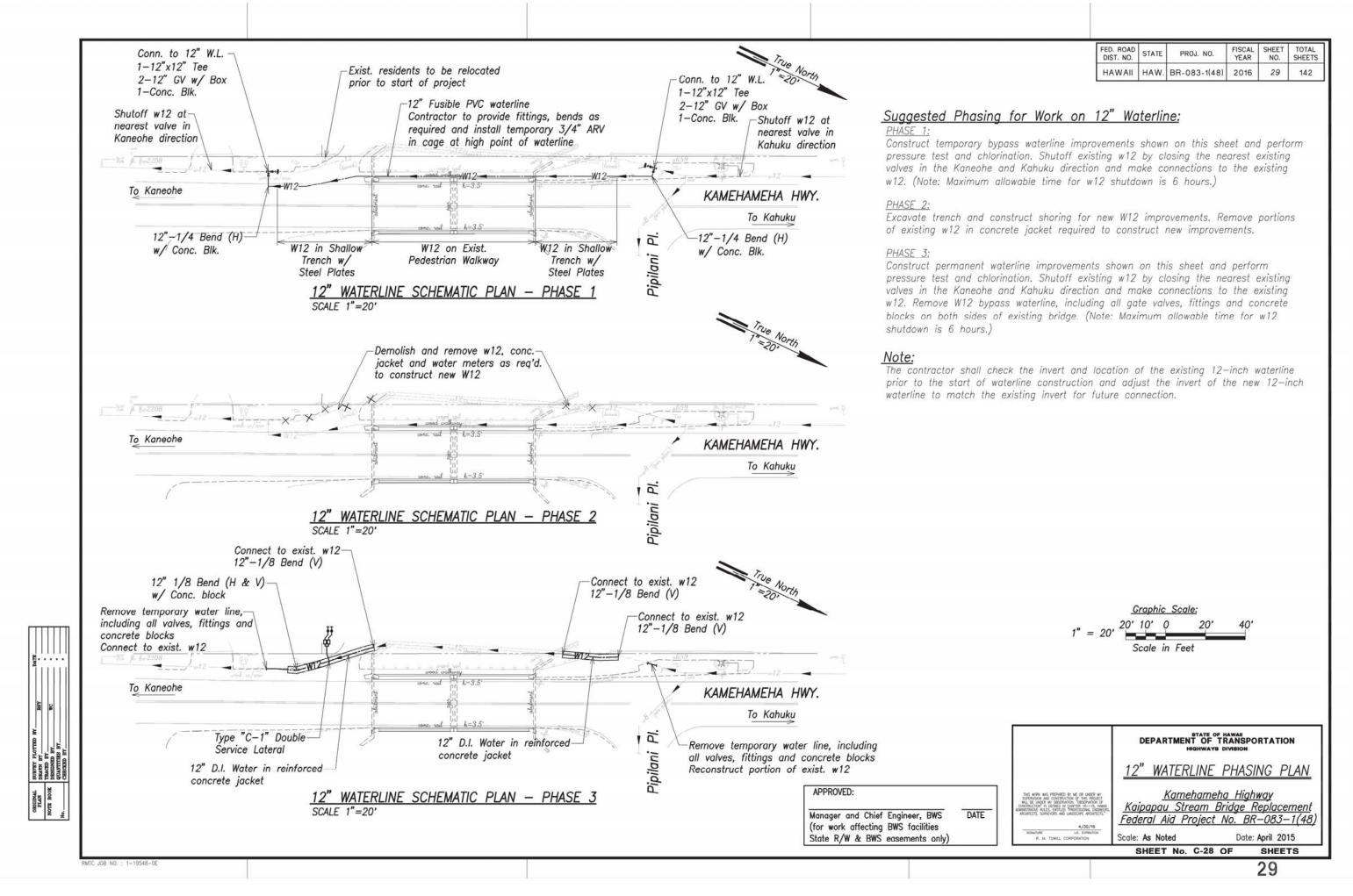


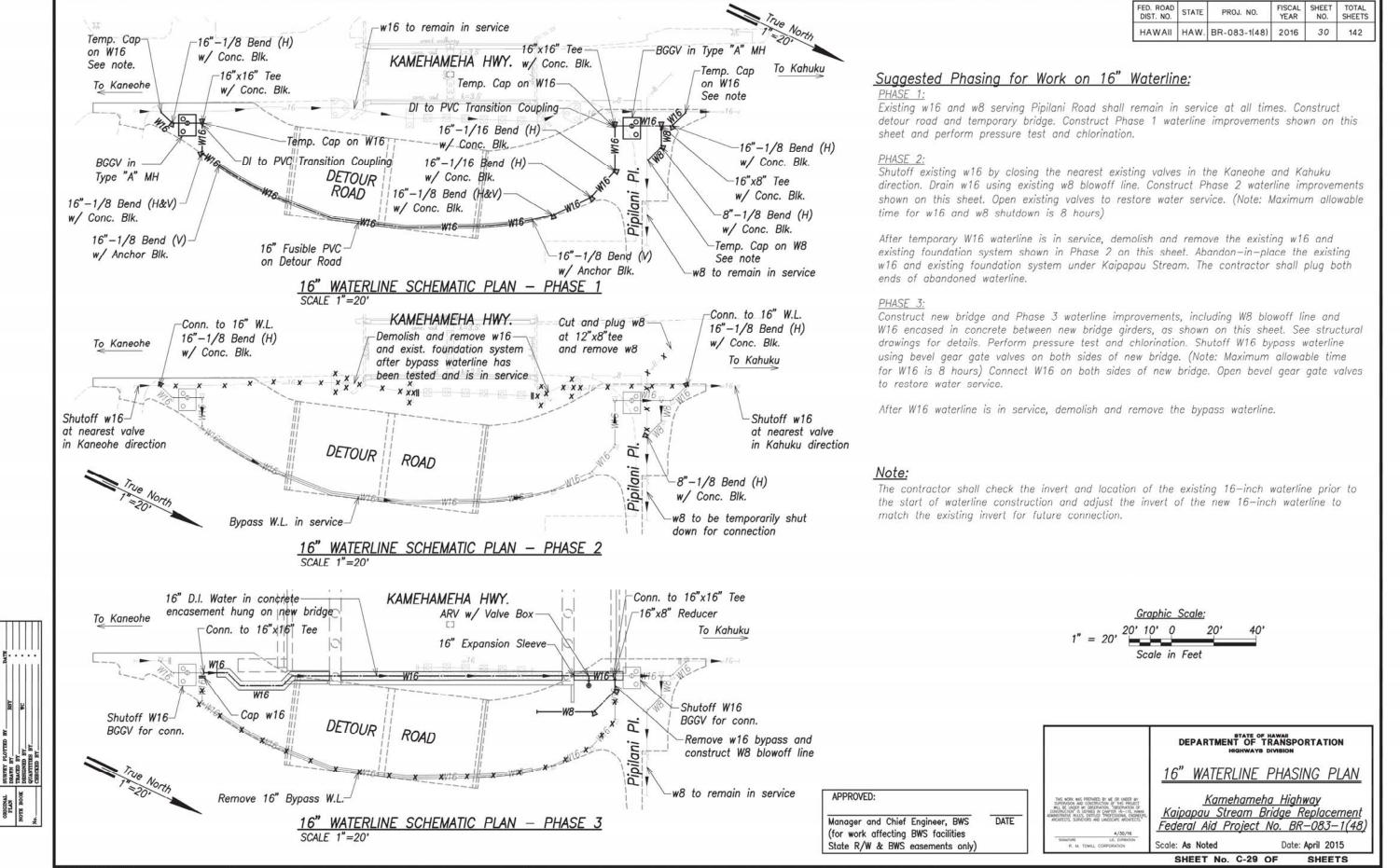


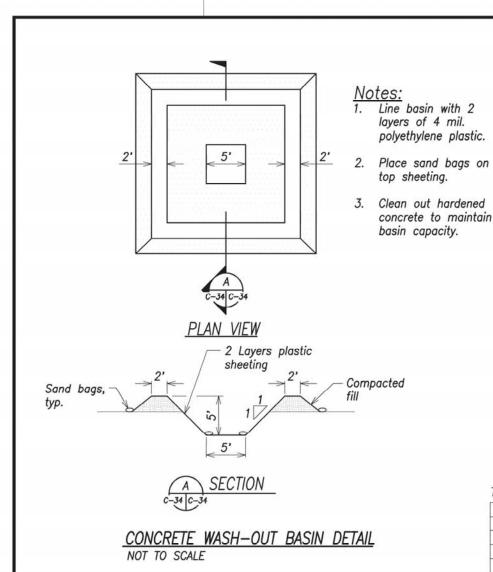


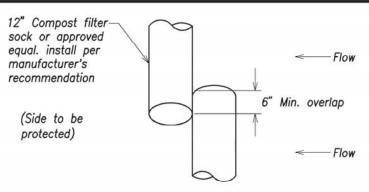


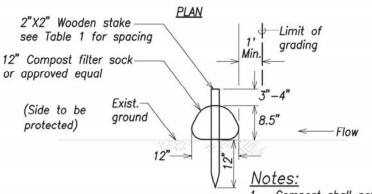








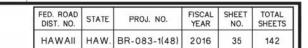


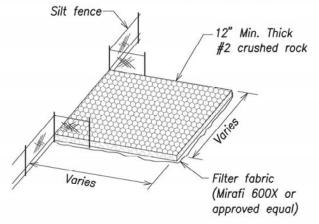


SECTION

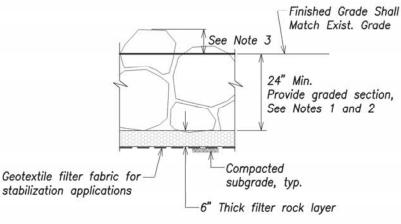
 Compost shall not contain biosolids and should be consistent with EPA guidelines as well as meet all local, state and federal quality requirements.

Contractor shall inspect compost filter socks when as required by the project SWPPP.





STABILIZED CONSTRUCTION ENTRANCE
Not To Scale



Notes

- Riprap shall be basalt rock and consist of 6"ø average stone size, 12"ø maximum stone size.
- 2. Riprap gradation:

 $D_{15} = 4$ "

 $D_{85} = 9"$ $D_{50} = 6"$

3. 6" Max. from highest adjacent rock finish top.

TEMPORARY DUMPED RIPRAP Not to Scale



DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

EROSION & SEDIMENT

CONTROL DETAILS

<u>Kamehameha Highway</u> <u>Kaipapau Stream Bridge Replacement</u> <u>Federal Aid Project No. BR-083-1(48)</u>

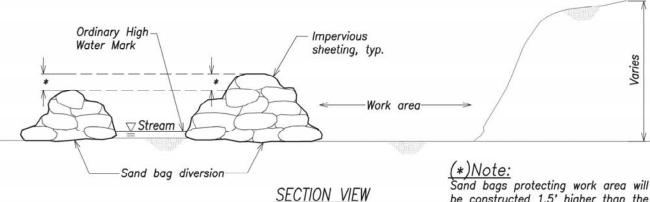
Scale: As Noted Date: April 2015

SHEET No. C-34 OF SHEETS

Table 1: Wooden stake anchor spacing

Siope	Anchor spacing					
< 4:1	Not required					
4:1 to 3:1	10' O.C.					
> 3:1 to 2:1	5' to 10' O.C.					
> 2:1	5' O.C.					

COMPOST FILTER SOCK DETAIL NOT TO SCALE



SAND BAG DETAIL
Not To Scale

Sand bags protecting work area will be constructed 1.5' higher than the opposite side to allow overflow of stream away from work area during high rainfall events.

SURYEY PLOTTED
DRAIN BY
TRACED BY
DESIGNED BY
QUANTITIES BY
CHECKED BY

To Kaneohe To Kahuku

110'-0"

5 8

CONSTRUCTION SEQUENCE

2'-10" Concrete

12

Barrier

STATE HAWAII HAW. BR-083-1(48) 2016 62

CONSTRUCTION SEQUENCE NOTES:

- 1. Order of construction sequence shall not be changed.
- 2. Each sequence stage shall be completely finished before proceeding to the next stage unless otherwise noted. The Engineer will be the sole judge of whether the sequence stage is complete, and may direct the Contractor to stop work on a sequence stage to complete work on the preceeding sequence stage.
- 3. Contractor shall submit overweight vehicular details for approval prior to their use.

LEGEND:

Phase 1 Stages

CONSTRUCTION SEQUENCE ELEVATION

Scale: 1/8" = 1'-0"

9

- Relocate existing utility lines.
- 2 Construct trial and load test shafts. Perform load test.

♠ Abutment No. 1

QuadGuard

18

- 3 Install detour road and temporary bridge.
- 4 Demolish existing bridge.

Approach Slab

14A

12 |

16

- 5 Construct precast girders. (May be done concurrently with Stages 1 through 4.)
- 6 Construct 4 ft diameter drilled shafts. Shaft numbers 1, 2, 3, 5, 6, 7.
- 7 Cast Phase 1 drilled shaft cap beams, girder seats, and corbels for concrete encased ducts at least 7 days after the final drilled shaft concrete pour in Stage 6 or until the concrete in Stage 6 has attained a compressive strength of 4,500 psi, whichever occurs later.
- 8 Erect Phase 1 precast girders at least 15 days after the concrete pour in Stage 7 or until the concrete in Stage 7 has attained a compressive strength of 5,000 psi, whichever occurs later. Place slush grout immediately prior to placement of precast girders.
- 9 Construct Phase 1 intermediate diaphragms.
- 10 Pour Phase 1 cast-in-place deck except areas over end beams and electrical duct encasement.
- 11 Pour Phase 1 corbel and end beams to top of precast girder at least 30 days after the concrete pour in Stage 10. The concrete pour shall occur between midnight and 3:00 AM (3 hour window).

Pour remainder of Phase 1 deck concrete a minimum of 24 hours after the concrete pour in Stage 11.

QuadGuard

18

Construct Phase 1 wing walls at least 8 days after the concrete pour in Stage 12 or after the concrete in Stage 12 has attained a compressive strength of 5,000 psi, whichever occurs

New Abutment Cap and

4'-0"\sqrt{Shafts}

- Backfill to Phase 1 limits and to bottom of approach slab at least 14 days after the concrete pour in Stage 13 or until the concrete in Stage 13 has attained a compressive strength of 5,000 psi, whichever occurs later. Maximum height difference of backfill between abutments shall not exceed 2 feet. Install concrete encased ducts behind abutments when backfill height is at the elevation of the bottom of the concrete encased electrical ducts. Continue backfilling after concrete for encased electrical ducts has attained its 28 day compressive strength.
- 14B Construct barrier wall.
- 15 Construct Phase 1 sleeper slabs.

¢ Abutment No. 2

- 16 Construct Phase 1 approach slabs.
- 17 Construct mauka aesthetic railing, concrete barrier and fence wall.
- 18 Install mauka quadguards.
- Install temporary barriers.



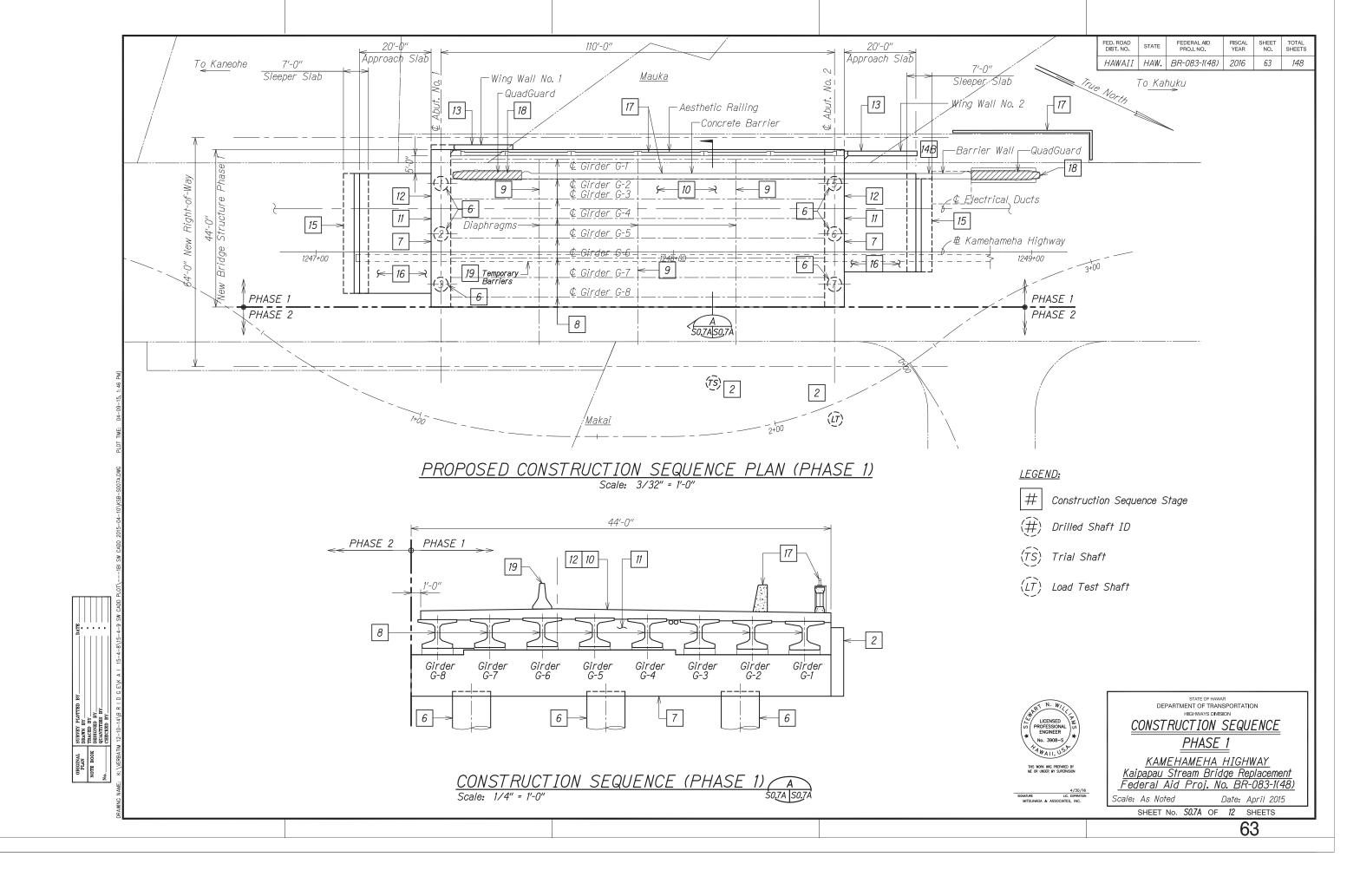
STATE OF HAWAPI
DEPARTMENT OF TRANSPORTATION CONSTRUCTION SEQUENCE PHASE 1

KAMEHAMEHA HIGHWAY Kaipapau Stream Bridge Replacement Federal Aid Proi. No. BR-083-1(48)

Scale: As Noted

Date: April 2015 SHEET No. SO.7 OF 12 SHEETS

SURVEY
DRAWN B
TRACED
DESIGNED
QUANTITIE



		KAIPAPAU STREAM BRIDGE	REPLAC	EMENT -	OVE	RALL	CONSTR	PUCTION	SEQU	'JENC	Œ			FE C
Struc Constru	uction	Description	0: "	1	rences			Waterline Work	Exist Op		Detour Open	Detour Off Peak Lane Closures	Remarks	
Sta Prior to Mobiliza for Dem	Site tion	 Prior to Site Mobilization, the Contractor shall submit required BMP's and other Municipal and National permit applications as indicated in the project Plans, Special Provisions and Specifications. The Contractor shall submit Prefabricated Steel Beam Bridge Structural Computations and Erection drawings to the Owner for Review and Approval Prior to Fabrication. 	Civil Civil Sequence See C-10. See Civil	Electrical	Geo	otech.	Structural Structural Sequence SO.7,SO.7A, SO.8,SO.8A	WOTK	Exist I Open I Traffic	Bridge to	Орен	Anticipated		
	1	1. Install approved BMP measures. 2. Relocate Existing overhead utility lines. 3. Install temporary 12" fusible PVC waterline on existing (upstream) pedestrian walkway.	C-15,16,17, C-20, C-28, see Civil 2	E-8, E-9, E-10, E-11				Temporary 12" fusible PVC waterline	9					
	2	1. Construct Trial and Load Test shafts * 2. Perform Load Test. Demobilize drilled shaft equipment off site.	See Civil 3		Special equipm		S1.1, S8.3						*Special Provisions Section 511	
	3	 Install Detour Pier, Abutments and Temporary Bridge. Construct Civil Phase 1 waterline Improvements C-29; C-30. Construct Detour Approach Retaining Wall, Fills and Roadway - chainlink fence see C-23. Construct Civil Phase 2 waterline improvements-see C-29; C-31. 	See Civil 4 C-23, C-29, C-30, C-31, C-32	E-10, E-11, E-15	Excava Bracing Prov.	ation g-Spec. 205*	S12.1, S12.2 S12.3, S12.4 S12.5	Civil Phase 1 & 2 (W16) waterline work—see C-29, C-30.			Detour Open to Traffic		*Excavation Bracing anticipated upstream of detour.	
	4	1. Relocate existing water line W12 (prior to existing bridge demolition) – see C-20, C-28. 2. Demolish existing bridge.	See Civil 5 C-20, C-28		Excava Bracing Prov. 2	ntion g-Spec. 205*	S2.1, S2.2	Relocate Exist W12 waterline C-20, C-28.					*Exc. Bracing upstream of existing.	
	5	Construct precast girders. (May be done concurrently with stages 1 through 4.)	See Civil 6				S4.x series							
	6	Construct 4 ft. diameter drilled shafts. 1, 2, 3, 5, 6, 7. *			Special equipm		S1.1,S1.2,S6.1, S6.2,S8.1,S8.2						*Special Provision Section 511	
	7	Cast phase 1 drilled shaft cap beams, girder seats, and corbels for concrete encased ducts at least 7 days after the final drilled shaft concrete pour in stage 6 or until the concrete in stage 6 has attained a compressive strength of 4,500 psi, whichever occurs later.			Structu Excava Bracing Spec F	tion g per Prov 205	S0.7, S0.7A, S6.x series						Marks 7 through 18 are PHASE 1 Structural see 2 for PHASE 2	h 1. 20
	8	Erect phase 1 precast girders at least 15 days after the concrete pour in stage 7 or until the concrete in stage 7 has attained a compressive strength of 5,000 psi, whichever occurs later. Place slush grout immediately prior to placement of precast girders.			Require Makai I		S0.7, S0.7A, S1.2, S1.3, S6 series	X						
	9	Construct phase 1 intermediate diaphragms.					S0.7,S0.7A, S5.x series							
	10	Pour phase 1 cast-in-place deck except areas over end beams and duct encasement.					S0.7,S0.7A S1.6,S3.1,S3.2							
1	11	Pour phase 1 end beams to top of precast girder and corbel at least 30 days after the concrete pour in Stage 10. The concrete pour shall occur between midnight and 3:00 AM (3 hours).					S0.7,S0.7A, S6.x series						Concrete Placement At Night	
PHASE	12	Pour remainder of phase 1 deck concrete a minimum of 24 hours after the concrete pour in stage 11.					\							
247	13	Construct phase 1 wing walls at least 8 days after the concrete pour in stage 12 or after the concrete in stage 12 has attained a compressive strength of 5,000 psi, whichever occurs later.					S0.7,S0.7A, S7.x series					Lane Closure Duration Approx 3 weeks	,	
131	14	Backfill to phase 1 limits and to bottom of approach slab and at least 14 days after the concrete pour in Stage 13 or until the concrete in Stage 13 has attained a compressive strength of 5,000 psi, whichever occurs later. Maximum height difference of backfill between abutments shall not exceed 2 feet. Install concrete encased ducts when backfill height is at the elevation of bottom of concrete encased ducts. Continue backfilling after concrete for encased ducts has attained its 28 day compressive strength.		Signal Corps Work E-1, E-5 E-12, E-13, E-16	5		S0.7,S0.7A,S6.x S9.x •					each abutment with Further Lane Closure Duration Approx 2 weeks each approach		
	15	Construct phase 1 sleeper slabs.												
	16	Construct phase 1 approach slabs.		Signal Corps Work E-1, E-5 E-12,E-13,E-1	16		\					\		
	17	Construct mauka aesthetic railings and concrete barrier.												
	18	Install mauka quadguards.												
	19	Install Temporary Barriers and Temporary Striping on PHASE I of New Bridge.	See Civil for Barriers										RT N. W//	

FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	BR-083-1(48)	2016	64	148

CONSTRUCTION SEQUENCE NOTES:

- Order of construction sequence shall not be changed unless authorized in writing by the Engineer.
- 2. Each sequence stage shall be completely finished before proceeding to the next stage unless otherwise noted. The Engineer will be the sole judge of whether the sequence stage is complete, and may direct the Contractor to stop work on a sequence stage to complete work on the preceeding sequence stage.
- 3. Contractor shall submit overweight vehicular details for approval prior to their use.
- 4. Construction shall be conducted such that no construction debris, wash water or other contaminants shall enter the Stream Waters.
- 5. Closing of the Prefabricated Steel Beam Bridge Structure:
 - (a) If for any reason or at any time, the Prefabricated Beam Bridge Structure's ability to safely carry traffic is in question, the Contractor shall be responsible for immediately taking the actions necessary to protect the public by closing, repairing and reopening the Prefabricated Steel Truss Bridge.

 When the Contractor closes the
 - (b) Prefabricated Steel Beam Bridge Structure, the Contractor shall immediately notify the Engineer and the appropriate Law Enforcement Agency. Closing of the Prefabricated Steel Beam
 - (c) Bridge shall be included as incidental to Maintenance of Traffic Control.
- 6. The Contractor shall phase 16 inch waterline (W16) to allow no more than 8 hours of down time. Liquidated Damages of \$100,000 per day will be imposed if the Contractor exceeds the 8 hour restriction.

LICENSED PROFESSIONAL OF ENGINEER No. 3908-S HOME WE PREPRISED BY ME OR UNDER MY SUPERISEON

4/30/16
SIGNATURE LIC. EXPIRATION
MITSUNAGA & ASSOCIATES, INC.

DEPARTMENT OF TRANSPORTATION
HIGHWAYS DIVISION

OVERALL CONSTRUCTION SEQUENCE

STRUCTURAL PHASE 1

<u>Kamehameha Highway</u> <u>Kaipapau Stream Bridge Replacement</u> <u>Federal Aid Project No. BR-083-1(48)</u>

Scale: AS NOTED

Date: April 2015

SHEET No. **S0.7B** OF **12** SHEETS **64**

To Kahuku

♠ Abutment No. 1 ♠ Abutment No. 2 Approach Slab 110'-0" 2'-10" Concrete Barrier (24)(28) QuadGuard (24)(28) *{25}*{*29*} (36) (26)(30) New Abutment Cap and 4'-0"\sqrt{Shafts}

CONSTRUCTION SEQUENCE

CONSTRUCTION SEQUENCE NOTES:

QuadGuard

(36)

- 1. Order of construction sequence shall not be changed.
- 2. Each sequence stage shall be completely finished before proceeding to the next stage unless otherwise noted. The Engineer will be the sole judge of whether the sequence stage is complete, and may direct the Contractor to stop work on a sequence stage to complete work on the preceeding sequence stage.
- 3. Contractor shall submit overweight vehicular details for approval prior to their use.

LEGEND:

Phase 2 Stages

To Kaneohe

CONSTRUCTION SEQUENCE ELEVATION

- Partially remove temporary bridge as required to construct Phase 2 of Kaipapau Stream Bridge
- Construct 4 ft diameter shafts Shaft nos. 4 and 8.
- Cast Phase 2 drilled shaft cap beams, girder seats, and corbels for concrete jacketed waterline at least 7 days after the final drilled shaft concrete pour in Stage 21 or until the concrete in Stage 21 has attained a compressive strength of 4,500 psi, whichever occurs later.
- Erect Phase 2 precast girders at least 15 days after the concrete pour in Stage 22 or until the concrete in Stage 22 has attained a compressive strength of 5,000 psi, whichever occurs later. Place slush grout immediately prior to placement of precast girders.
- Construct Phase 2 intermediate diaphragms between girders G-9 and G-10, install dowels connecting G-10 and G-11, and install W16 with light-weight concrete jacket between girders G-10 and G-11.
- Pour Phase 2 cast-in-place deck except areas over end beams and closure pour.
- Pour Phase 2 corbel and end beams (except at closure pour) to top of precast girder at least 30 days after the concrete pour in Stage 25. The concrete pour shall occur between midnight and 3:00 AM (3 hour window).
- Pour remainder of Phase 2 deck concrete (except at closure pour) a minimum of 24 hours after the concrete pour in Stage 26.
- Pour Phase 2 intermediate diaphragms between girders G-8 and G-9 at least 4 days after the concrete pour in Stage 27.

- Pour Phase 2 cast-in-place deck closure except over end beams. Material for cast-in-place deck closure pour shall be VESLMC. (See Special Provisions).
- Pour Phase 2 corbel and end beam closure from top of drilled shaft cap beam to top of deck. Material for end beam closure pour shall be VESLMC. (See Special Provisions).
- Construct Phase 2 wind walls at least 8 days after the concrete pour in Stage 30 or after the concrete in Stage 30 has attained a compressive strength of 5,000 psi, whichever occurs later.
- Backfill to bottom of approach slab at least 14 days after the concrete pour in Stage 31 or until the concrete in stage 31 has attained a compressive strength of 5,000 psi, whichever occurs later. Maximum height difference of backfill between abutments shall not exceed 2 feet. Install jacketed waterline behind abutments when backfill height is at the elevation of the bottom of the jacketed waterline. Continue backfilling after concrete for jacketed waterline has attained its 28 day compressive strength.
- Construct Barrier Wall.
- Construct Phase 2 sleeper slabs.
- (34) Construct Phase 2 approach slabs.
- Constuct Makai aesthetic railing and concrete barrier.
- (36) Install Makai quadquards.
- Remove remainder of temporary bridge.



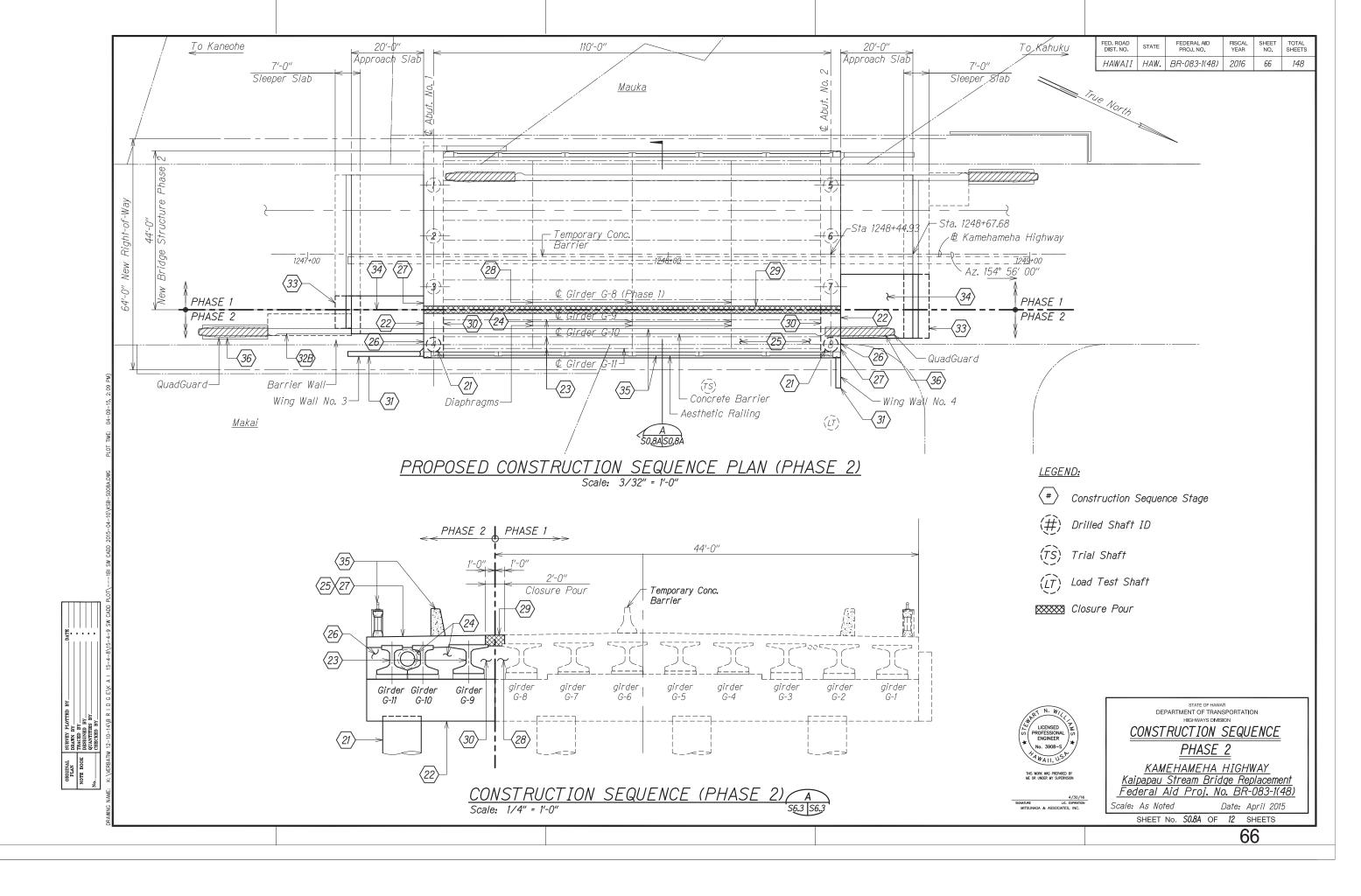
STATE OF HAWAPI
DEPARTMENT OF TRANSPORTATION

CONSTRUCTION SEQUENCE PHASE 2

KAMEHAMEHA HIGHWAY Kaipapau Stream Bridge Replacement Federal Aid Proi. No. BR-083-1(48)

Scale: As Noted

Date: April 2015 SHEET No. SO.8 OF 12 SHEETS



		KAIPAPAU STREAM BRIDGE	REPLACI	EMENT —	OVE	PALL	CONSTR	UCTION	SEQU	JENO	CE		244.5		FI
Struct Construc	tion	Description	Civil		rences	44	C4m. a.4. may	Waterline Work	Exist Op		Detour Open	I lane Ci	Off Peak Iosures	Remarks	
Stage	29	 Open PHASE I of New Bridge to traffic. Close Temporary Bridge and Detour Roadway to traffic. Remove Mauka portion of Temporary Bridge Only (Remainder to remain in place to support construction equipment for construction of PHASE 2 portion of New Bridge and to support temporary W16 until Final W16 is constructed). 	Civil	Electrical	Geot	есп.	Structural S0.8, S0.8A	non.	PHASE New Bi Open to Traffic allow L Closure	l of ridge to to Detour	Close Detour and Remove Limited Portion of Temporary Bridge			Close Detour; Open PHASE 1 of New Bridge: Start Construction of PHASE 2 of New Bridge	
(2	?1>	Construct 4 ft. diameter drilled shafts - Shaft nos. 4 and 8.	See Civil 6		Special equipme	drilling ent*	S1.1, S1.2, S6.1 S6.2, S8.1, S8.2	,			Detour Close	d		*Special Provisions Section 511	
(2	<i>!2</i> >	Cast Phase 2 drilled shaft cap beams, girder seats, and corbels for concrete jacketed waterline at least 7 days after the final drilled shaft concrete pour in stage 21 or until the concrete in stage 21 has attained a compressive strength of 4,500 psi, whichever occurs later.			Structur Excavati Bracing Spec for Required Approac	re ion per r 205 d at	S0.8, S0.8A, S6.x series							Special Provisions Section 205	
4	<u>'</u> 3>	Erect Phase 2 precast girders at least 15 days after the concrete pour in stage 22 or until the concrete in stage 22 has attained a compressive strength of 5,000 psi, whichever occurs later. Place slush grout immediately prior to placement of precast girders.			*	,	S0.8, S0.8A, S1.2, S1.3, S6.x series	Civil Phase 3 (W16) waterline improvements seeC-29,C32							
(2		Construct Phase 2 intermediate diaphragms between girders G-9 and G-10 and light-weight W16 concrete jacket between girders G-10 and G-11.	C-29, C-30				S0.8,S0.8A, S5.x series								
(2	25	Pour Phase 2 cast-in-place deck except areas over end beams and closure pour.					S0.8,S0.8A S1.6,S3.1,S3.2								1
2		Pour Phase 2 end beams (except at closure pour) to top of precast girder and corbel at least 30 days after the concrete pour in Stage 25. The concrete pour shall occur between midnight and 3:00 AM (3 hours).					S0.8,S0.8A, S6.x series							Concrete Placement At Night	
PHASE	?Z	Pour remainder of Phase 2 deck concrete (except at closure pour) a minimum of 24 hours after the concrete pour in stage 25.													
	28 >	Pour Phase 2 intermediate diaphragms between girders G-8 and G-9 at least 4 days after the concrete pour in stage 27.													
STRUCTURAL		Pour Phase 2 cast-in-place deck closure except over end beams. Material for cast-in-place deck closure pour shall be VESLMC.													
		Pour Phase 2 end beams closure from top of drilled shaft cap beam to top of deck. Material for end beam closure pour shall be VESLMC.													
(2		Construct Phase 2 wing walls at least 8 days after the concrete pour in stage 30 or after the concrete in stage 30 has attained a compressive strength of 5,000 psi, whichever occurs later.					S0.8,S0.8A, S7.x series								
	<i>32</i> >	Backfill to bottom of approach slab at least 14 days after the concrete pour in Stage 31 or until the concrete in Stage 31 has attained a compressive strength of 5,000 psi, whichever occurs later. Maximum height difference of backfill between abutments shall not exceed 2 feet. Install jacketed waterline when backfill height is at the elevation of bottom of the jacketed waterline. Continue backfilling after concrete for jacketed waterline has attained its 28 day compressive strength.					S0.8,S0.8A,S6.x S9.x								
Q.	IJ\	Construct Phase 2 sleeper slabs.													1
(3	34	Construct Phase 2 approach slabs.													
(3	<i>5</i>	Construct Makai aesthetic railings and concrete barrier.													1
	76	Install Makai guadguards. Remove Detour; construct stream hardening. Remove Temporary Barriers at New Bridge. Open Phase 1 and Phase 2 of New Bridge to traffic.	See Civil 7 thru 12	Permanent Electrical Plan See E-12,E-13 E-14	7,			Remove temp W16 at Closed Detour	PHASE PHASE New Bi Open	2 of	Remove Remainder of Detour				

FED. ROAD DIST. NO.	STATE	PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
HAWAII	HAW.	BR-083-1(48)	2016	67	148

CONSTRUCTION SEQUENCE NOTES:

- 1. Order of construction sequence shall not be changed unless authorized in writing by the
- 2. Each sequence stage shall be completely finished before proceeding to the next stage unless otherwise noted. The Engineer will be the sole judge of whether the sequence stage is complete, and may direct the Contractor to stop work on a sequence stage to complete work on the preceeding sequence stage.
- 3. Contractor shall submit overweight vehicular details for approval prior to their use.
- 4. Construction shall be conducted such that no construction debris, wash water or other contaminants shall enter the Stream Waters.
- 5. Closing of the Prefabricated Steel Beam Bridge Structure:
 - (a) If for any reason or at any time, the Prefabricated Beam Bridge Structure's ability to safely carry traffic is in question, the Contractor shall be responsible for immediately taking the actions necessary to protect the public by closing, repairing and reopening the Prefabricated Steel Truss Bridge.
 - (b) When the Contractor closes the Prefabricated Steel Beam Bridge Structure, the Contractor shall immediately notify the Engineer and the appropriate Law Enforcement Agency.
 - (c) Closing of the Prefabricated Steel Beam Bridge shall be included as incidental to Maintenance of Traffic Control.
- 6. The Contractor shall phase 16 inch waterline (W16) to allow no more than 8 hours of down time. Liquidated Damages of \$100,000 per day will be imposed if the Contractor exceeds the 8 hour restriction.

No. 3908-S THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION

4/30/16

SIGNATURE LIC. EXPIRATION
MITSUNAGA & ASSOCIATES, INC.

DEPARTMENT OF TRANSPORTATION

OVERALL CONSTRUCTION SEQUENCE

STRUCTURAL PHASE 2

<u>Kamehameha Highway</u> Kaipapau Stream Bridge Replacement Federal Aid Project No. BR-083-1(48)

Scale: AS NOTED SHEET No. **SO.8B** OF **12** SHEETS

Date: April 2015

RMTC JOB NO.: 1-19548-0E 67 Attachment A-3: Quantity of Storm Water Discharge Calculations

KAIPAPAU STREAM BRIDGE REPLACEMENT (PROJECT NO. BR-083-1(48)) KAMEHAMEHA HIGHWAY, STATE ROUTE 83

Storm Event: 10-year, 1-Hr. rainfall recurrence interval

Hydrologic Criteria: The hydrologic criteria established in the City and County of Honolulu, Department of Planning and Permitting, Storm Drainage Standards (2000), will be utilized in calculating design flows.

Method of Determining Existing Design Flows

Design flows for the developed areas will be based on the Rational Method (for drainage areas less than 100 acres):

Q = Flow rate (cfs), where

C = Runoff coefficient

I = Rainfall intensity in inches per hour for a duration equal to the time of

concentration (inches/hr)

A = Drainage area in acres

Hydrologic Calculations

Rational Method (Drainage Areas Less Than 100 Acres):

Runoff Coefficient, C

C = 0.90 for flat paved areas

Average Rainfall Intensity, I

I = 2.2 in/hr (Plate 1: Intensity of 10-year, 1-Hr. Rainfall)

Time of Concentration, Tc

Tc = 7.5 min. (Plate 3: Overland Flow Chart)

Correction Factor, F

F = 2.50 (Plate 4: Correction Factor)

Drainage Area, A

A = 1.6 acres

Q = (0.90)(2.2 in/hr)(2.50)(1.6 acres) = 7.92 cfs

REFERENCES

Rules Relating to Storm Drainage Standards, Department of Planning and Permitting, City & County of Honolulu, January 2000.

Table 2 MINIMUM RUNOFF COEFFICIENTS FOR BUILT-UP AREAS

RESIDENTIAL AREAS:

C = 0.55 to 0.70

HOTEL-APARTMENT AREAS: C

= 0.70 to (0.90)

BUSINESS AREAS:

C = 0.80 to 0.90

INDUSTRIAL AREAS:

C = 0.80 to 0.90

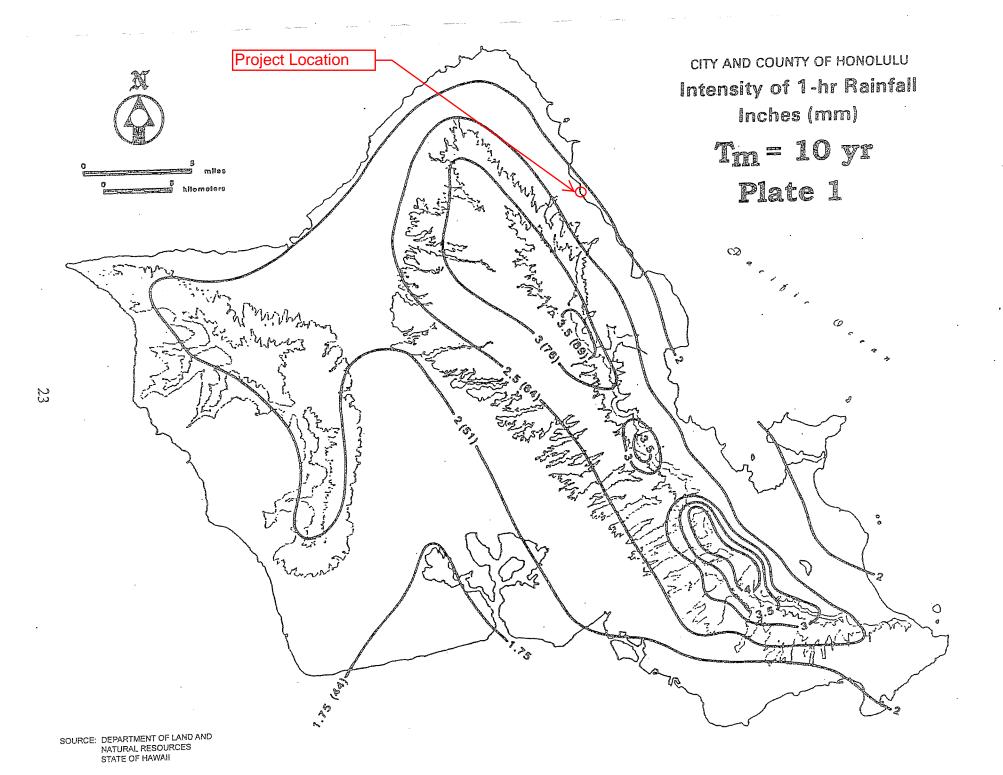
The type of soil, the type of open space, and ground cover and the slope of the ground shall be considered in arriving at reasonable and acceptable runoff coefficients.

Table 3

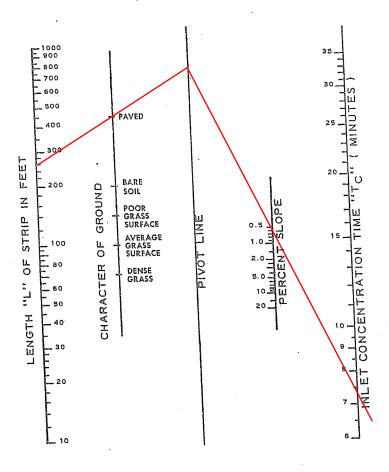
APPROXIMATE AVERAGE VELOCITIES OF RUNOFF FOR CALCULATING TIME OF CONCENTRATION

TYPE OF FLOW	VELOCITY IN fps FOR SLOPES (in percent) INDICATED
OVERLAND FLOW: Woodlands Pastures Cultivated Pavements	0-3% 4-7% 8-11% 12-15% 1.0 2.0 3.0 3.5 1.5 3.0 4.0 4.5 2.0 4.0 5.0 6.0 5.0 12.0 15.0 18.0
OPEN CHANNEL FLOW: Improved Channels Natural Channel* (not well defined)	Determine Velocity by Manning Formula 1.0 3.0 5.0 8.0

^{*} These values vary with the channel size and other conditions so that the ones given are averages of a wide range. Wherever possible, more accurate determinations should be made for particular conditions by Manning Formula or from Plate 5.







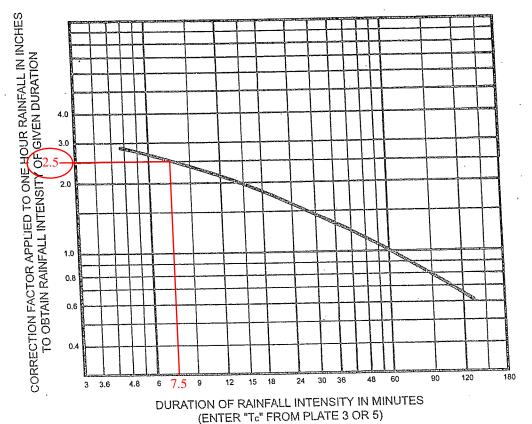


Plate 4

CORRECTION FACTOR

FOR CONVERTING 1 HR. RAINFALL TO RAINFALL INTENSITY OF VARIOUS DURATIONS

> TO BE USED FOR AREA LESS THAN 100 ACRES

(See Plate 6 for area more than 100 acres)

Total Runoff: Storm Water Runoff 7.92 cfs (See Attachment A-3: Quantity of Strom Water Discharge Calculations) Site-Specific Best Management Practices (BMPs) Plan 7.92 cfs (See Attachment A-2: Construction Plans/Drawings) A. Management Controls - Good housekeeping practices - Keep adjacent road area free of dirt and mud - Cover construction material and debris as applicable B. Structural Controls - Stream diversion - Wall makai of bridge for slope protection - Rip-rap or CRM slope protection at the abutment walls - Dewatering basin - Concrete wash-out basin C. Vegetative Controls - Grass/mulch all slopes and exposed areas Receiving State Waters 7.92 cfs

Discharge Point 1 & 2 Kaipapa'u Stream

Class 2 Inland

7.92 cfs